

TRAFFIC AND PARKING IMPACT ASSESSMENT

MIXED USE PLANNING PROPOSAL

815 PACIFIC HIGHWAY, CHATSWOOD & 15 HELP STREET, CHATSWOOD



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1 INTRODUCTION

M^cLaren Traffic Engineering (MTE) was commissioned by *A*+ *Design* Group to prepare a Traffic and Parking Impact Assessment of the proposed Mixed Use Planning Proposal at 815 Pacific Highway, Chatswood & 15 Help Street, Chatswood. The relevant plans and schedule of areas are reproduced in **Annexure A** for reference.

1.1 Description and Scale of Development

The planning proposal includes the amalgamation of two sites (815 Pacific Highway & 15 Help Street) and the implementation of a mixed-use development comprising retail and commercial and basement car parking. The proposed building has a height up to 142m above ground level. The proposed mixed-use development consists of the following scale relevant to traffic and parking and assuming approval of the proposal, would comprise the following:

- Café 312m² GFA;
- Retail 2,755m² GFA;
- Commercial Office 57,457m² GFA.

Vehicular access to the proposed development will be available through a single two-way driveway from McIntosh Street. The first basement level will provide some car parking, but also loading facilities for vehicles up to an 8.8m length Medium Rigid Vehicle (MRV). Multiple levels of basement parking are provided below basement 1.

1.2 State Environmental Planning Policy (Infrastructure) 2007

If the planning proposal were approved, the subsequent development application would qualify as a traffic generating development with relevant size and/or capacity under Clause 104 of the SEPP (Infrastructure) 2007. Formal referral to the Roads and Maritime Services (RMS) would be necessary.

1.3 Site Description

1.3.1 Existing Conditions

The subject site situated at 815 Pacific Highway, Chatswood has a total area of approximately 1,657m² and at 15 Help Street, which has an approximate site area of 1,863m². The site is surrounded by a mix of commercial, residential and retail developments with generally higher density developments along The Pacific Highway and lower density residential developments to the west of site. The site at 815 Pacific Highway fronts Pacific Highway on its western boundary and gains vehicular access via a two-way driveway on the southern boundary to Help Street. The site at 15 Help Street is bounded by commercial developments and gains vehicular access via a two-way on the northern boundary to McIntosh Street.

The subject site is currently zoned B3 – Commercial Core Use under the *Willoughby LEP* 2012 (WLEP).

1.4 Site Context

The location of the site is shown on aerial imagery and on a map in **Figure 1** and **Figure 2** and respectively.



815 Pacific Highway

15 Help Street





E 815 Pacific Highway

FIGURE 2: SITE CONTEXT – STREET MAP

2 EXISTING TRAFFIC AND PARKING CONDITIONS

2.1 Road Network

The road network surrounding the site has the following characteristics within close proximity to the site:

Pacific Highway

- RMS Classified STATE Road (HW 10);
- Approximately 22m wide carriageway facilitating 3 lanes in each direction and a raised median, separating traffic flow directions with breaks in the median at intersections;
- Signposted 60km/h speed limit;
- Various parking zones including "1-hour parking" between 8:30am to 3pm Mon to Fri and 8:30am to 12:30pm Saturday, and "No Stopping" signs along the site's frontage.

Help Street

- Unclassified LOCAL Road;
- Approximately 14m wide carriageway facilitating two traffic lanes in both directions;
- Signposted 40km/h speed limit due to high pedestrian activity area;
- Signposted "*No Parking*" on both sides of the street near the site frontage, time restricted parking to the east of the site frontage.

Victoria Avenue

- Unclassified LOCAL Road;
- Approximately 13m wide carriageway facilitating two traffic lanes in both directions;
- Signposted 40km/h speed limit due to high pedestrian activity area;
- Time restricted parking along both sides of the street between 9:30am-3:30pm on weekdays and 8:30am-4:30pm on Saturdays, unrestricted parking on Sundays.

Railway Street

- Unclassified LOCAL Road;
- Approximately 12m wide carriageway facilitating two traffic lanes in both directions;
- Signposted 40km/h speed limit due to high pedestrian activity area;
- Bus Zones along the eastern side of the street, time restricted parking along the west side of the street between 9:30am-6pm on weekdays and 8:30am-4:30pm on Saturdays, unrestricted parking on Sundays.

McIntosh Street

- Unclassified LOCAL Road;
- Approximately 10m wide carriageway facilitating one traffic lanes in each direction;
- Signposted 40km/h speed limit due to high pedestrian activity area;
- Signposted "*Motorbike Parking*" on both sides of the street near the site frontage, time restricted parking to the east of the site frontage.

2.2 Existing Traffic Environment

Traffic counts were completed at the following intersections of Chatswood during the AM and PM peak periods on Thursday 15th November 2018.

- Pacific Highway / Help Street / Fullers Road
- Pacific Highway / Victoria Avenue
- Help Street / Railway Street

Counts at the above intersections were undertaken prior to the involvement of the site at 15 Help Street, Chatswood. Once this site was included, MTE engaged traffic counts at the following intersections during both the AM and PM peak periods on Thursday 27th July 2019.

- Railway Street / McIntosh Street
- Pacific Highway / McIntosh Street
- Pacific Highway / Railway Street

It is evident from the traffic surveys that the AM peak hour within the surrounding road network occurs between 7:15am-8:45am, whilst the PM peak hour occurs at some hour period between 3:45pm-5:30pm. The peak hour differs slightly between intersections. As a conservative assessment, each intersections' peak volumes are assumed to occur concurrently. Full results of the traffic surveys are reproduced in **Annexure B**.

2.3 Existing Traffic Management

- Signal controlled intersection of Pacific Highway / Help Street / Fullers Road;
- Signal controlled intersection of Help Street / Railway Street;
- Signal controlled intersection of Pacific Highway / Victoria Avenue;
- Signal controlled intersection of Pacific Highway / Railway Street;
- Signal controlled intersection of McIntosh Street / Railway Street.

2.3.1 Intersection Performances

Existing intersection performances have been assessed using SIDRA INTERSECTION 7.0, the results of this analysis are summarised in **Table 1**, with full results reproduced in **Annexure C**.

TABLE 1: EXISTING INTERSECTION PERFORMANCES (SIDRA INTERSECTION 8)

Intersection	Peak Hour	Degree of Saturation ⁽¹⁾	Average Delay ⁽²⁾ (sec/veb)	Level of Service ⁽³⁾⁽⁴⁾	Control Type	Worst Movement	95th Percentile Queue	
			EXISTING P	ERFORMANC	E			
		0.00	0.8	NA		RT from Site	0.3 veh (2.2m)	
Pacific Hwy	AM	0.20	(Worst: 11.9)	(Worst: A)	Give	Road	Help Street	
Site / Help St		0.00	0.7	NA	Way	RT from Site	0.3 veh (2.1m)	
	РМ	0.30	(Worst: 15.1)	(Worst: B)		Road	Site Road	
		0.07	16.4	В		RT from Help	4.5 veh (33m)	
Railway St /	АМ	0.67			0. 1	Street	Help Street	
Help St	-	0.00	17.9	В	Signals	RT from Help	5.1 veh (36.7m)	
	РМ	0.63				Street	Help Street	
	0.04	0.02	2.7	NA		RT from Site	0 veh (0.2m)	
Help St Site /	AIVI	0.03	(Worst: 5.7)	(Worst: A)	Give	Road	Site Road	
McIntosh St	DM	0.00	1.4	NA	Way	RT from Site	0.1 veh (0.7m)	
P	PIVI	0.06	(Worst: 5.9)	(Worst: A)		Road	Site Road	
	АМ	0.22	2.1	NA		RT from	0.7 veh (4.8m)	
Railway St / Mcintosh St		0.22	(Worst: 7.9)	(Worst: A)	Give	Street	Railway Street	
	PM	0.18	2.1	NA	vvay	RT from Mcintosh	0.7 veh (4.6m)	
			(Worst: 8.5)	(Worst: A)		Street PT from	Mcintosh Street	
Pacific Hwy /	AM	0.69	11.8	A	Give	Victoria Avenue	14.8 veh (106m) Pacific Highway	
Victoria Ave	PM	0.65	9	Α	Way	RT from Victoria	9.7 veh (70.3m)	
						Avenue	Pacific Highway	
	AM	AM 0.93	0.93	35.2	С		T from Help	12.3 veh (91.5m)
Pacific Hwy / Fullers Rd					Give Way	Sileet	Pacific Highway	
	PM	0.83	21.7	В	may	RT from Fullers Road	13.1 veh (95.2m) Pacific Highway	
			3.9	N/A		LT from	19.7 veh (140.9m)	
Pacific Hwy /	АМ	0.44	(Worst: 7.4)	(Worst: A)	Give	Mcintosh Street	st: Pacific Highway	
McIntosh St	РМ	0.46	3.7	N/A	Way	LT from st: Pacific	1 veh (7.2m) st: Pacific	
			(Worst: 6.5)	(Worst: A)		Highway	Highway 21.2 yob	
	AM	0.92	25.1	В		LT from	(223.3m)	
Railway St / Pacific Hwv					Signals	raliway Street	Highway	
	PM	0.81	12.3	A		LT from Railway Street	20.3 veh (144.2m)	

NOTES:

(1) Degree of Saturation is the ratio of demand to capacity for the most disadvantaged movement.

(2) The average delay is the delay experienced on average by all vehicles. The value in brackets represents the delay to the most disadvantaged movement.

(3) The level of Service is a qualitative measure of performance describing operational conditions. There are six levels of service, designated from A to F, with A representing the best operational condition and level of service F the worst. The LoS of the intersection is shown in bold, and the LoS of the most disadvantaged movement is shown in brackets.

(4) NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

As shown, the relevant intersections are currently performing at an acceptable level of efficiency, all with a level of service "A", "B", or "C" conditions in both the AM & PM peak hours. The level of service "A" performance is characterised by low approach delays and spare capacity.

2.4 Public Transport

The subject site is very well located for public transport accessibility based on current provision. It is known that there will be new provisions in the medium term with the likely approval of the Chatswood Metro Station some 350m southeast of the site.

Existing public transport accessibility of the site is described by:

- 350m from Chatswood Railway Station with bi-directional servicing at 5 to 10-minute intervals throughout each weekday on the T1 Northern Line, connecting the site in all cardinal directions to commercial, recreation and residential uses and the wider public transport network.
- Within 400m walking distance to Chatswood Station Bus Stops with bi-directional services at 1 to 5-minute intervals throughout each weekday, connecting the site in all cardinal directions to commercial, recreation and residential uses and the wider public transport network. The site is serviced by bus routes 100, 143, 144, 200, 255, 256, 258, 259, 261, 530, 533, 534, 536, 558, 565, 627, 628, N90, N91, SL1, SL2, 21T1, 22T1, 23T1, and 24T1. A bus stops on the western side of the Pacific Highway is located approximately 120m to the south of the subject site and is therefore highly accessible for pedestrians.

The public transport network map within closer proximity to the site is shown in **Figure 3** below





FIGURE 3: CONTEXT MAP – NETWORK MAP

2.5 Future Infrastructure Changes

2.5.1 Western Harbour Tunnel and Beaches Link

The NSW Government is currently in the planning process for the Western Harbour Tunnel and Beaches Link project which provides a link between the Western Suburbs and the Northern Beaches. The Western Harbour Tunnel is expected to create a western bypass of the Sydney CBD, and more specifically the Sydney Harbour Bride and Tunnel. The proposal will change the way people move around Sydney and how people travel between the north Shore, CBD and western and southern Sydney. The indicative plan is shown in **Figure 4** below.



FIGURE 4: WESTERN HARBOUR TUNNEL

2.5.2 Sydney Metro – Chatswood Station

As part of the eight new metro railway stations currently under construction as part of the Sydney Metro Northwest (Stage 1, due to open in the first half of 2019), Chatswood will be provided with a Metro Train Station as part of the Stage 2 works (expected to open in 2024), connecting Chatswood to Sydenham. Further, the Sydney Metro will be connected between Sydenham to Bankstown. The project will provide Metro trains between Rouse Hill, Chatswood, North Sydney, Sydney CBD, Sydenham and Bankstown. The facility will provide high frequency trains to major employment, retail and education centres for the region. **Figure 5** below indicates the extent of the Sydney Metro project.



FIGURE 5: SYDNEY METRO NORTHWEST LINK

Chatswood Station will provide access to the Sydney Metro Line. The Sydney Metro line will reduce private car dependency for residents of the subject proposal and therefore will lessen the traffic impact on the surrounding road network, by providing alternative modes of travel for residents and visitors.

3 PARKING ASSESSMENT

The proposal will generate demand for parking by residents, visitors and staff. Estimation of the likely demand for parking at the site should be considered in conjunction with local planning policies, contextual effects such as regional location and individual land uses. For the purpose of the assessment, reference is made to the Willoughby Council Development Control Plan 2016 (WDCP).

3.1 Parking Quantum

3.1.1 Council Parking Requirement

Reference is made to Willoughby Council's DCP 2016: Chapter C.4 which outlines a range of controls that need to be considered in terms of design and implementation of off-street parking facilities to support developments within Willoughby Council area. The relevant parking requirements are outlined below:

Office/Business premises within Railway Precincts (besides those mentioned below) and Major Public Transport Corridors as defined in Clause C.4.1-E

1 / 110m²

Office/Business premises in Chatswood (Zone B3 under <u>WLEP 2012</u>) where access is only available from Pacific Highway, Albert Avenue, Victoria Avenue, Help or Railway Streets

1/200m²

Shops

1 space / 25m²

Restaurants (within railway precincts)

1 space per 25m² plus 1 space per 2 employees

Where the amount of parking required is not a whole number, the number of spaces required will be rounded down to the nearest whole number.

The existing site at 815 Pacific Highway has access only to Help Street. Therefore, the parking rate of 1 per 200m² applies to this site in the existing condition. However, the amalgamated site proposes access off McIntosh Street. It follows that the parking rate of 1 per 110m² applies to the proposed site by strict application of the WDCP. The above car parking requirements are summarised in **Table 2** below.

Land Use	Туре	Scale	Rate	Parking Provision
Commercial	Office	57,457m ²	1 per 110m ²	522.3
Retail	Specialty Shop	2,755m ²	1 per 25m ²	110.2
		312m ²	1 per 25m ²	12.5
Restaurant	Cafe	10	Plus 1 per 2 employees	5
Total				650

TABLE 2: COUNCIL DCP CAR PARKING REQUIREMENTS

As shown above, based upon Council's DCP car parking requirements the proposed development is required to provide a minimum of **650** total car parking spaces (522 commercial office car parking spaces and 128 retail car parking spaces).

3.1.2 Retail and Café Parking Requirement

The WDCP rate of 1 space per 25m² is intended for a standalone site and does not specify the location of the site in terms of zoning. The site is located within the Chatswood CBD area; thus, the retail portion of the development is intended to serve the existing locality only and no visitor parking should be provided. Visitors to the ground floor retail use are expected to already be in the surrounding area and will access the site on foot or via public transportation.

The cafés should therefore provide staff parking at a rate of 1 space per 2 employees. It is expected that there will be a maximum of 10 staff on site at any one time between the two sites, resulting in the requirement of five (5) spaces.

The retail tenancy should only provide staff parking at a rate of 1 space per 80m², which results in a parking requirement of 35 spaces between the two sites. Applying a rate of 1 space per 2 staff results in a maximum of 70 staff on site at any one time, which is considered more than reasonable to operate the retail premises.

Therefore, the retail and staff components of the proposal should only require a total of **40** spaces (5 for café, and 35 for retail).

3.1.3 Commercial Office Parking Requirement

The WDCP rate of 1 space per 200m² is intended for office/business premises in Chatswood where access is only available from Pacific Highway, Albert Avenue, Victoria Avenue, Help of Railway Streets. The intention of this reduced rate is to limit the volume of traffic traveling to / from sites within Chatswood CBD along the heavily trafficked roads referenced within the quoted parking requirement.

Although the proposed access point is off McIntosh Street, the site is bounded by Pacific Highway and Help Street. Vehicles traveling to the site must utilise the Pacific Highway, Help Street, Railway Street and/or Victoria Avenue within very close proximity to the site.

Chatswood is a "Strategic Centre" as defined by *A Plan for Growing Sydney*. Therefore, the intention of the Chatswood CBD is to provide jobs and transport (including the North West Rail Link) for the surrounding community. It would be beneficial to the objectives of the B3 zone and the strategic centre to promote public transportation usage and limit vehicular traffic to the site where possible, in accordance with the WDCP rate for office/business premises in the Chatswood CBD.

The site is very well serviced by public transportation being located within Chatswood CBD approximately 300m walking distance to Chatswood Train Station and 175m walking distance to the Chatswood Bus Interchange. With consideration for the above, it is appropriate to apply the parking requirement of 1 space per 200m², in accordance with the WDCP rate for office/business premises in Chatswood CBD.

3.1.4 Recommended Parking Requirement

Taking the above parking rate analysis into consideration, the recommended car parking requirement is outlined in **Table 3** below.

Land Use	Туре	Scale	Rate	Parking Provision
Commercial	Office	57,457m ²	1 per 200m ²	287.3
Retail	Specialty Shop	2,755m ²	1 per 80m ²	34.4
Restaurant	Cafe	10 staff	1 per 2 employees	5
Total				327

TABLE 3: RECOMMENDED CAR PARKING REQUIREMENT

As shown above, the recommended car parking requirement is **327** spaces, including 287 commercial spaces, 35 retail staff spaces and 5 café staff retail spaces. The subject proposal shall provide sufficient parking to satisfy the above demand in accordance with the relevant Australian Standards.

3.2 Disabled Parking

The WDCP does not provide disabled parking rates for commercial and retail developments, thus the disabled parking requirement is taken from the Building Code of Australia (BCA). Based upon the BCA, office buildings require the provision of 1% of all parking to be provided as disabled parking, resulting in a requirement of four (4) disabled spaces.

Retail / shopping centre developments require the provision of 2% of all parking to be provided as disabled parking, resulting in a requirement of one (1) disabled space. Detailed design of the basement car parking for the provision of disabled car parking and parking provision will be undertaken during DA stage.

3.3 Servicing & Loading

Council's DCP stipulates the following servicing / loading requirements applicable to the subject development:

Off-street loading and unloading facilities must be provided for all businesses, commercial, office and retail as required by Council. The size and number of loading bays provided will be determined by Council having regard to the:

- Frequency of deliveries;
- Size and bulk of goods;
- Size of trucks;
- Availability of on-street loading zones; and
- Intended use of the premises.

Based upon the above requirements and the proposed scale, a loading bay sufficient for a Medium Rigid Vehicle (MRV) would be adequate to cater for the proposed development. The proposed plans include a ramp which provides satisfactory access for MRVs to the basement 1 level. The plans must provide a minimum 4.5m headroom clearance for MRVs, as specified by *AS2890.2:2018*. If this is not practical, an alternative loading vehicle can be specified at the DA stage. The detailed design of waste collection and delivery vehicle access into the basement will be undertaken at the DA stage, although, a typical swept path arrangement is provided in **Annexure D**.

3.4 Bicycle and Motorcycle Parking

Council outlines the following bicycle parking rates applicable to the proposed development.

Commercial Office Lockers - 1 per 600m² GFA Rail / Racks – 1 space per 2500m² GFA Retail / Restaurant Lockers - 1 per 450m² GFA

Rail / Racks – 1 space per 150m² GFA

The above bicycle parking rates are summarised in **Table 4** below.

Land Use	Туре	Scale	Rate	Bicycle Spaces Required
Commercial	Lockers	57 157m2	1 per 600m ²	96
	Rail / Racks	J7,4J7111⁻	1 per 2,500m ²	23
Retail - Shops	Lockers	0.007.2	1 per 450m ²	7
	Rail / Racks	3,067m²	1 per 150m ²	20
Total	Lockers	-	-	103
	Rail / Racks	-	-	43

TABLE 4: DCP BICYCLE PARKING REQUIREMENTS

As shown above, the site requires the provision of **103** bicycle lockers for staff, and **43** bicycle rails / racks for visitors based upon the requirements of Council's DCP which shall be provided to promote sustainable transport methods.

Bicycle facilities are to be provided in accordance with the 3 classes set out in the DCP (Class 1 for residents, Class 2 for staff/employees and Class 3 for visitors of any land use).

Willoughby Council also specifies a generic motorcycle parking rate for non-residential developments with a minimum provision of 1 space per 25 cars or part thereof. Applying this rate, and adopting a parking provision of 327, **13-14** motorcycle spaces are required for the proposed development.

As per the DCP requirements, showers and change facilities must be provided where more than three bicycle lockers are provided and must be located close to the secure bicycle parking.

3.5 Access Arrangements

The proposal represents the amalgamation of two (2) adjacent sites and the elimination of one (1) access point from Help Street. All access to the site will be provided via a two-way driveway ramp connecting the site to McIntosh Street along the northern frontage. The elimination of the Help Street access driveway will improve both pedestrian and vehicular traffic flows along the Help Street corridor, which is one of the highest trafficked roadways in the Chatswood CBD.

3.6 Car Park Design & Compliance

The car parking layout and access thereto, shall be assessed for compliance against the relevant sections of *AS2890.1:2004*, *AS2890.2:2002* and *AS2890.6:2009*. A detailed assessment of this requirement is expected to be undertaken during detailed DA stage. The design is to achieve the following:

• **Parking Aisle width**: Minimum 5.8 metres. However, it is recommended that all ramp widths which service visitor parking exceed the minimum width requirement to facilitate circulation.

- **Ramp widths**: A minimum of 6.1m width is to be provided along ramps between walls and a minimum of 5.5m between kerbs. Where heavy vehicle access is required (to the first basement level), the ramp should be wide enough to accommodate passing of the heavy vehicle and a B85.
- **Parking bays:** Minimum 2.4m width and a minimum 5.4m bay length for employees. A 300mm widening of the parking space is required for each side wall obstruction for car door opening effects. A minimum parking bay length of 5.4 metres is required, unless a small bay which can have the dimensions of 5.0 metres long by 2.3 metres wide
- Access Driveway Widths: A driveway servicing between 301-600 Class 1 and 1A (i.e. all day parkers including employees) parking spaces from a local road is a Category 3 driveway. Therefore, the driveway must be 10-12m in width, and separated exit and entry by 1-3m. Justification of a reduced access driveway width can be considered on a case by case basis based upon the traffic generation of the site.
- Driveway Gradient for User Class 1 and 1A (i.e. all-day parkers including employees): To satisfy sight lines to pedestrians on footpaths and to comply with under carriage clearance and overhang checks. In this regard driveways need to achieve acceptable performance as follows:
 - Provide a 2m by 2.5m sight triangle upon departure to the road boundary to provide adequate sight lines to pedestrians. See Figure 3.3 of AS2890.1-2004.
 - 1:20 for the first 6 metres from the property boundary however this can be reduced if necessary based on individual assessments.
 - **Headroom**: Minimum of 2.2m EXCEPT for the area directly above disabled parking spaces (including shared spaces) where minimum headroom of 2.5m is required in accordance with Clause 2.4 of AS2890.6:2009.
- Driveway Gradient for Heavy Vehicles (including delivery and waste collection): To satisfy sight lines to pedestrians on footpaths and to comply with under carriage clearance and overhang checks. In this regard driveways need to achieve acceptable performance as follows:
 - Provide a 2m by 2.5m sight triangle upon departure to the road boundary to provide adequate sight lines to pedestrians. See Figure 3.3 of AS2890.1-2004.
 - Max 1:6.5 gradient.
 - Maximum rate of change for heavy vehicles as below:
 - SRV: 1:12 (8.3%) in 4.0m of travel
 - MRV: 1:16 (6.25%) in 7.0m of travel
 - HRV: 1:16 (6.25%) in 10m of travel.
 - **Headroom**: Minimum of 3.5m for SRVs, and 4.5m for MRVs and HRVs.

- **Shared Zone**: A shared zone must be located adjacent to the parking bay on either side. A Shared zone must also be included at the front and rear of the parking bay. It should be noted that the aisle can be deemed a shared zone for the front or rear of the parking bay. Where a shared zone separates two parking bays or separates the disabled space and a wall, a Bollard is to be installed in accordance to *Figure 2.3* of *AS2890.6 2009*.
- **Disabled Space:** Non-residential disabled car parking spaces are to be designed in accordance with AS2890.6:2009, requiring a 2.4m wide spaces by 5.4m in length with adjacent shared space of the same dimensions.

The detailed design and review of the basement carpark is subject to further DA assessment.

4 TRAFFIC ASSESSMENT

The impact of the expected traffic generation levels associated with the subject proposal is discussed in the following sub-sections.

4.1 Commercial Office Traffic Generation

Reference is made to the RMS Guide Technical Direction (TDT2013/04a) which provides the following average generation rates for commercial office buildings.

AM Peak – 1.6 per 100m² GFA

PM Peak – 1.2 per 100m² GFA

4.1.1 RMS Surveys

These rates are based off surveys from ten (10) office buildings in the Sydney Metropolitan Area which were completed in 2010. One of the surveyed sites is an office building located at 9 Help Street, Chatswood, which is approximately 75m walking distance from the subject site. It is reasonable to assume that the subject proposal will operate similarly to the surveyed site at 9 Help Street, Chatswood. The 2010 peak hour traffic generation data for the site is:

AM peak hour - 1.03 trips / 100m²;

PM peak hour – 0.84 trips / 100m².

4.1.2 Journey to Work Data

The above rates are based on surveys from 2010. Reference is made to Journey to Work Data, which is summarised in **Table 5**, and shown in full in **Annexure E**.

Mode of Transport	2011	2016
Car as driver	51.21%	43.67%
Train	29.39%	33.83%
Bus	5.48%	6.35%
Car as passenger	4.26%	3.38%

TABLE 5: JOURNEY TO WORK DATA – CHATSWOOD CBD

The data between 2011 and 2016 shows a decline in reliance on private vehicles. The percentage of employees who drive to work in the Chatswood CBD area dropped from 51.21% in 2011 to 43.67% in 2016. Assuming a linear relationship between trip generation rates and car driver percentage, the following traffic generation rates apply based on the 2016 JTW data.

AM peak hour – 0.88 trips / 100m²;

PM peak hour - 0.72 trips / 100m².

There were no major public transportation projects completed between 2011 and 2016, thus it is reasonable to assume that the decline in private vehicle usage is a natural trend and

will likely continue in the future. The addition of the Sydney Metro service to Chatswood Station in 2024 is likely to further reduce reliance on private vehicles. The rates above can therefore be considered conservative traffic generation rates in the future.

4.1.3 Existing Site Traffic

The existing retail use on the site would be expected to generate traffic according to the RMS Guide Technical Direction (TDT2013/04a), whilst the existing commercial office use would be expected to generate traffic according to the rates calculated in **Section 4.1.2**.

Retail:

Shops - 5.6 trips per 100sqm of GFA in the Friday PM peak hour

Commercial:

AM Peak - 0.88 per 100m² GFA

PM Peak - 0.72 per 100m² GFA

The existing traffic generation of each existing site is summarised in Table 6.

Land Use	Scale	Rate ⁽¹⁾⁽²⁾⁽³⁾⁽⁴⁾⁽⁵⁾ (Trips per 100m ²)		Genera	ted Trips			
		AM	AM PM		РМ			
815 Pacific Highway								
Office	6 367 4m ²	0.88	0.72	-56	-45			
Onice	0,007.411	0.00	0.72	(45 in, 11 out)	(9 in, 36 out)			
Retail	$400m^{2}$	56	5.6	-22	-22			
Tetali	40011	5.0	5.0	(11 in, 11 out)	(11 in, 11 out)			
Subtotal				-78	-67			
Subiotal				(56 in, 22 out)	(20 in, 47 out)			
		15 He	lp Street					
Office	5 334m ²	0.88	0.72	-47	-39			
	0,00411	0.00	0.72	(38 in, 9 out)	(8 in, 31 out)			
Rotail	73m ²	56	5.6	-4	-4			
Retail	7.5111	5.0	5.0	(2 in, 2 out)	(2 in, 2 out)			
Subtotal				-51	-43			
Oubiotal				(40 in, 11 out)	(10 in, 33 out)			
Total				-129	-110			
Total				(96 in, 33 out	(30 in, 80 out)			

TABLE 6: EXISTING TRAFFIC GENERATION

Notes:

(1) AM Retail traffic generation is conservatively assumed to be 50% of the PM generation rate.

(2) PM Café traffic generation is conservatively assumed to be 50% of the AM generation rate

(3) Residential assumes 20% inbound & 80% outbound during AM peak for commercial: Vice versa for PM.

(4) Commercial assumes 90% inbound & 10% outbound during AM peak for commercial: Vice versa for PM.

(5) Retail traffic split of 50% inbound and outbound during the AM and PM period.

As shown, the existing buildings on site generate a total of 129 trips and 110 trips in the AM and PM peak hours respectively. This traffic has been discounted from the proposed traffic generation to estimate the additional trips along the road network for the proposal.

4.1.4 Additional Site Traffic

The estimated traffic generation level for the proposed mixed-use development, based upon the RMS Guide Technical Direction (TDT2013/04a) and the calculated commercial rates is shown below:

Commercial

AM Peak - 0.88 per 100m² GFA

PM Peak – 0.72 per 100m² GFA

Retail

Shops - 5.6 trips per 100sqm of GFA in the Friday PM peak hour

Restaurant

Café – 5 trips per 100sqm of GFA

A summary of the traffic generation is shown in **Table 7** below.

Land Use	Rate ⁽¹⁾⁽²⁾⁽³⁾⁽⁴⁾⁽⁵⁾ Scale (Trips per 100m ²)		Generated Trips		
		АМ	РМ	AM	РМ
		Proposed T	raffic Gen	eration	
Office	57,457m ²	0.88	0.72	+506 (455 in, 51 out)	+414 (41 in, 373 out)
Retail	2,755m ²	5.6	5.6	+154 (77 in, 77 out)	+154 (77 in, 77 out)
Café	312m ²	5	2.5	+16 (8 in, 8 out)	+8 (4 in, 4 out)
Subtotal				+676 (540 in, 136 out)	+576 (122 in, 454 out)
		Existing T	raffic Gene	ration	
Office 815 Pacific Hwy	5,334m ²	0.88	0.72	-47 (42 in, 5 out)	-38 (4 in, 34 out)
Office 15 Help Street	6,367.4m ²	0.88	0.72	-56 (50 in, 6 out)	-46 (5 in, 41 out)
Retail 815 Pacific Hwy	400m ²	5.6	5.6	-22 (11 in, 11 out)	-22 (11 in, 11 out)
Retail 15 Help Street	73m ²	5.6	5.6	-4 (2 in, 2 out)	-4 (2 in, 2 out)
Subtotal				-129 (105 in, 24 out)	-108 (21 in, 87 out)
Total				+547 (435 in, 112 out)	+468 (101 in, 367 out)

TABLE 7: ESTIMATED TRAFFIC GENERATION

Notes:

(1) AM Retail traffic generation is conservatively assumed to be 50% of the PM generation rate.

(2) PM Café traffic generation is conservatively assumed to be 50% of the AM generation rate

(3) Residential assumes 20% inbound & 80% outbound during AM peak for commercial: Vice versa for PM.

(4) Commercial assumes 90% inbound & 10% outbound during AM peak for commercial: Vice versa for PM.

(5) Retail traffic split of 50% inbound and outbound during the AM and PM period.

As shown above the proposed planning proposal is expected to generate 676 vehicle trips (540 in, 136 out) during the AM peak hour period and 576 vehicle trips (122 in, 454 out) in the PM peak hour period. Discounting the existing traffic generation from the existing development results in an additional **547** trips (435 in, 112 out) during the AM and **468** vehicle trips (101 in, 367 out) in the PM peak hour period.

It is noted that the standard café and retail traffic generation rates have been applied to the subject development. These rates include both staff and visitor vehicular traffic. The café and retail sites should not be provided with any dedicated visitor car parking spaces,

therefore the traffic generation rates used in **Table 7** are expected to be overestimated, resulting in a conservative traffic assessment.

4.2 Traffic Distribution

Both site accesses are serviced by two-way driveways allowing both right turns and left turns. Traffic will arrive to the site from the north/south along the Pacific Highway, from the west along Fullers Road, and from the east along Help Street/McIntosh Street. The assignment of the additional traffic generated by the site will follow that of the existing trip percentage splits taken from the existing traffic surveys. A diagram showing the traffic distribution throughout the surrounding road network is provided in **Annexure F**.

4.3 Traffic Impact

The traffic generation outlined in **Section 4.1** and **4.2** above has been added to the existing traffic volumes recorded. SIDRA INTERSECTION 8 was used to assess the intersections performance. The purpose of this assessment is to compare the existing intersection operations to the future scenario under the increased traffic load. The results of the existing SIDRA assessment are shown in **Table 8**, whilst the future SIDRA results (post development) are shown in **Table 9**. Full results of the future SIDRA analysis for the proposed access arrangement is reproduced in **Annexure G**:

TABLE 8: EXISTING INTERSECTION PERFORMANCES (SIDRA INTERSECTION 8)

Intersection	Peak Hour	Degree of Saturation ⁽¹⁾	Average Delay ⁽²⁾ (sec/veb)	Level of Service ⁽³⁾⁽⁴⁾	Control Type	Worst Movement	95th Percentile Queue			
	EXISTING PERFORMANCE									
			0.8	NA		RT from Site	0.3 veh (2.2m)			
Pacific Hwv	AM	0.20	(Worst: 11.9)	(Worst: A)	Give	Road	Help Street			
Site / Help St			0.7	NA	Way	RT from Site	0.3 veh (2.1m)			
	РМ	0.30	(Worst: 15.1)	(Worst: B)		Road	Site Road			
	0.04	0.50	27.6	В		RT from Help	8.6 veh (61m)			
Railway St /	AIVI	0.50			Cignolo	Street	Help Street			
Help St	DM	0.55	27.5	В	Signals	RT from Help	10 veh (72.3m)			
		0.55				Street	Help Street			
	ΔΜ	0.03	2.7	NA		RT from Site	0 veh (0.2m)			
Help St Site /		0.05	(Worst: 5.7)	(Worst: A)	Give	Road	Site Road			
McIntosh St		DM	0.06	1.5	NA	Way	RT from Site	0.1 veh (0.7m)		
		0.00	(Worst: 5.9)	(Worst: A)		Road	Site Road			
	AM	0.22	2.1	NA		RT from Mcintosh	0.7 veh (4.8m)			
Railway St /			(Worst: 7.9)	(Worst: A)	Give	Street	Railway Street			
Montosti Ot	PM	0.18	2.1	NA		RT from Mcintosh	0.7 veh (4.6m)			
			(Worst: 8.5)	(Worst: A)		Street RT from	Mcintosh Street			
Pacific Hwy /	AM	0.69	11.8	A	0. 1	Victoria Avenue	14.8 veh (106m) Pacific Highway			
Victoria Ave	РМ	0.65	9	Α	Signals	RT from Victoria Avenue	9.7 veh (70.3m) Pacific Highway			
	AM	0.93	34.6	С		T from Help	12.3 veh			
Pacific Hwy /		AW	Pacific Hwy /	0.95			Signals	Street	Pacific Highway	
	РМ	0.83	21.7	В		RT from Fullers Road	13.1 veh (95.2m) Pacific Highway			
	ΔМ	0.44	3.9	N/A		LT from Mcintosh	20.7 veh (148.3m)			
Pacific Hwy / McIntosh St		0.77	(Worst: 7.5)	(Worst: A)	Give Way	Street	st: Pacific Highway			
Montoon ot	PM	0.46	3.7	N/A	vvay	LT from st: Pacific	1 veh (7.2m) st: Pacific			
			(Worst: 6.5)	(Worst: A)		Highway	Highway 22.5 veh			
Railway St /	AM	0.83	12.6	A	Signals	LT from Railway Street	(160.8m) st: Pacific Highway			
Pacific Hwy	PM	0.81	12.3	A	<u> </u>	LT from Railway Street	20.3 veh (144.2m)			

Notes: See notes for Table 1

TABLE 9: FUTURE INTERSECTION PERFORMANCES (SIDRA INTERSECTION 8)

Intersection	Peak Hour	Degree of Saturation ⁽¹⁾	Average Delay ⁽²⁾ (sec/veh)	Level of Service ⁽³⁾⁽⁴⁾	Control Type	Worst Movement	95th Percentile Queue		
FUTURE PERFORMANCE – Proposed Arrangements (Post Development)									
Poilwov St /	AM	0.83	34.4	с		RT from Railway Street	11.4 veh (81.2m) Help Street		
Help St	РМ	0.75	28	В	Signals	RT from Help Street	10.1 veh (72.9m) Help Street		
Help St Site /	AM	0.28	5.4 (Worst: 7)	NA (Worst: A)	Give	RT from Site Road	0.5 veh (3.4m) Site Road		
McIntosh St	PM	0.40	5.1 (Worst: 6.5)	NA (Worst: A)	Way	RT from Site Road	1.9 veh (13.4m) Site Road		
Railway St /	AM	0.39	4.8 (Worst: 11)	NA (Worst: A)	Give	RT from Mcintosh Street	3 veh (21.4m) Railway Street		
Mcintosh St	РМ	0.47	4.3 (Worst: 10.7)	NA (Worst: A)	Way	RT from Mcintosh Street	3.1 veh (21.4m) Mcintosh Street		
	AM	0.90	14.4	А		RT from Pacific Highway	15.1 veh (108.3m) Pacific Highway Railway Street		
Victoria Ave	PM	0.70	8.6	A	Signals	RT from Victoria Avenue	10.3 veh (74.4m) Pacific Highway Mcintosh Street		
Pacific Hwy /	AM	0.97	38.4	С	Gianala	T from Help Street	13 veh (96.8m) Pacific Highway		
Fullers Rd	РМ	0.79	22.8	В	Signais	RT from Fullers Road	13.7 veh (99.8m) Pacific Highway		
Pacific Hwy /	AM	0.44	4 (Worst: 7.2)	N/A (Worst: A)	Give	LT from Mcintosh Street	22.3 veh (160m) st: Pacific Highway		
McIntosh St	PM	0.53	3.9 (Worst: 7.6)	N/A (Worst: A)	Way	LT from Mcintosh Street	2.1 veh (14.9m) st: Pacific Highway		
Railway St /	АМ	0.89	19.4	В		LT from Railway Street	29.8 veh (213.1m) st: Pacific Highwav		
Pacific Hwy	PM	0.84	15	В	Signals	LT from Railway Street	21.4 veh (152.6m) st: Pacific Highway		

Notes: See notes for Table 1

As shown in **Table 8** and **Table 9** above, the nearby intersections remain relatively unaltered under the future scenario with the additional traffic generated by the proposal. The existing driveway for 815 Pacific highway has been eliminated from the future analysis. The existing LoS has generally been retained with the intersections experiencing minimal delays and spare capacity. All changes to intersection LoS are acceptable and are summarised in **Table 10**.

Intersection	Peak Hour	Existing Level of Service / Avg. Delay	Future Level of Service / Avg. Delay
Railway Street / Help Street	AM	B / 27.6 sec	C / 34.4 sec
Railway Street / Pacific Highway	AM	A / 12.6 sec	B / 19.4 sec
	PM	A / 12.3 sec	B / 15 sec

TABLE 10: CHANGES TO LEVEL OF SERVICE

The addition of the development traffic results in only moderately low increases to the average delay at the nearby intersections. Changes in LoS from "A" to "B" and "B" to "C" are acceptable changes, therefore, the proposal does not adversely impact the surrounding road network. The proposal is supported in terms of its potential traffic impacts from a traffic flow efficiency and a road safety perspective.

4.4 Potential for Reduction in Traffic Generation

Although the traffic generated by the proposal has not been found to have an adverse impact on the surrounding road network, it is beneficial to outline potential traffic generation reduction strategies which could be employed at the DA stage. These strategies include, but are not limited to, the discussion within the following subsections.

4.4.1 Cyclist/Active Transport Facilities

Cyclists often cite safety, lack of facilities and lack of familiarity with an area as reasons to avoid commuting via bicycle. The subsequent design could include a safe and convenient access to the bike storage area, which is well signposted. End of trip facilities may include bike servicing tools, towel service, spacious change facilities, lockers, bubblers and laundry services to encourage alternative transport to the site. An increase in cyclist / active transport would theoretically reduce the traffic generation of the proposal.

4.4.2 Journey to Work Data

In **Table 5**, MTE demonstrated that "car as driver" commuters have decreased between the 2011 and 2016 census data. A subsequent DA, if undertaken after the 2021 census, could investigate how this trend has continued after 2016. Should the reduction in "car as driver" commuter mode of transport continue, the expected traffic generation would likely be less than what is estimated in **Table 7**.

4.4.3 Limitations to Car Parking Provision

Although the Council's DCP outlines a rate of 1 space per 110m² for this site, and 1 space per 200m² for sites with direct access to particular streets within the Chatswood CBD, Willoughby Council has the ability to support a lesser car parking provision should they wish to further reduce the traffic generation potential of the subject proposal.

5 CONCLUSION

In view of the foregoing, the planning proposal for a mixed-use development (as depicted in **Annexure A**) is supportable in terms of its traffic and parking impacts subject to the provision of the required compliant parking, and further DA assessment of the traffic impact on the surrounding road network. The following outcomes of this traffic impact assessment are relevant to note:

- The proposed development would require **327** car parking spaces for the development based upon Council's DCP and a reasonable assessment of retail / commercial parking demand within the Chatswood CBD.
- A subsequent compliant development application would require approximately **103** secure bicycle lockers and **43** visitor bicycle rails / racks according to Council DCP rates.
- **13-14** motorcycle spaces are required as a part of the development.
- A subsequent compliant development application would generate a net maximum of 547 movements in the AM (435 in, 112 out) peak hour and 468 trips in the PM (101 in, 367 out) peak hour.
- The proposed development at 815 Pacific Highway and 15 Help Street requires the provision of multiple 8.8m length Medium Rigid Vehicle loading bays for waste collection within the basement.
- Council and the BCA requires that a minimum of 2% the retail spaces and 1% of the commercial spaces must be disabled spaces. Given the scale of the development, a total of five (5) disabled parking spaces are required, four (4) for the commercial portion, and (1) for the retail portion. Disabled spaces are to be designed in accordance with AS2890.6.
- The car park layout is to be designed in accordance with AS2890.1:2004, AS2890.6:2009 and AS2890.2002. The detailed design of the basement car park will be reviewed during the development application stage.



ANNEXURE A: PROPOSED PLANS

(5 SHEETS)



WWW.APLUSDG.COM.AU | NOMINATED ARCHITECT: TONY LEUNG NSW 7133









815 Pacific Hwy, CHATSWOOD

Job: a18088 | Date: 22 JULY 2019

† G

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(SHEET 1 OF 6)



HELP STREET / RAILWAY STREET / PACIFIC HIGHWAY

(SHEET 2 OF 6)

Peak T	ime	North /	Approach	n Railway	/ Street	East	Approa	ch Help S	treet	South	n Approacl	h Railway	Street	Wes	t Approa	ch Help S	street	Peak
Period Star P	Period End	U	R	SB	L	U	R	WB	L	U	R	NB	L	U	R	EB	L	total
07:45	08:45	0	79	156	100	0	68	197	184	0	84	81	67	0	7	326	108	1457
15:45	16:45	0	165	113	106	0	57	280	95	0	133	110	102	0	8	277	46	1492
Note: Site ske	etch is for ill	ustrating	traffic flow	ws. Direct	ion is indi	cative only	r, drawing	is not to s	scale and	not an exa	act streets	configuration	on.					
Graphic																		
Total			Railway S	treet						Ra	ilway Street							
Light Heavy 	10 16 107 26 108			13 1 143 99 156 100		orth		eet 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			13 152 100 165 113	3 103 106	North	- -				
H Heb			eak 7:45 AN	1-8:45 AM		4 9 8 193 175 8 197 184	lo Street	Help Str 0 3 3 2 2	ک. دک	PM Peak	3:45 PM-4:4	5 PM	5 270 83 7 280 95	p Street				
		67 45 22	81 8 78 7 3 8	4 0 6 0 8 0						102 1 74 1 28	10 133 05 125 5 8	0 0 0						

RAILWAY STREET / HELP STREET

(SHEET 3 OF 6)

Peak	Time	North A	pproach	Pacific H	lighway	East A	pproach	Victoria /	Avenue	South	Approach	Pacific Hi	ghway		West App	roach N//	4	Peak
Period Star	Period End	U	R	SB	L	U	R	WB	L	U	R	NB	L	U	R	EB	L	total
07:15	08:15	0	0	2135	55	0	58	0	75	0	159	1417	5	0	1	0	1	3906
15:00	16:00	0	0	1296	63	0	79	0	89	0	136	1958	1	0	4	1	4	3631
Note: Site sl	ketch is for ill	lustrating	traffic flow	vs. Direct	ion is india	cative only	, drawing	is not to s	scale and	not an exa	act streets	configuratio	on.					
Graphic																		
Total			Pacific Hig	hway						Pac	ific Highway							
Light Heavy	کے م	>		63 26 072 29 135 55	N N N N N	orth	Vintoria	A 0 1 - 0 4			0 49 0 1247 0 1296		North	0 Victoria				
Ž			eak 7:15 AN	-8:15 AM	$\langle \neg \rangle$	0 28 0 47 0 75		0 0 T	ج ک		3:00 PM-4:0	<	;_ ° °	Avenue				
		5 4 1	1417 18 1320 19 97 9	59 0 50 0 9 0							58 136 85 127 3 9	0 0 0						
			Pacific High	iway						Pac	ific Highway							

VICTORIA AVENUE / PACIFIC HIGHWAY

(SHEET 4 OF 6)



RAILWAY STREET / MCINTOSH STREET

(SHEET 5 OF 6)



MCINTOSH STREET / PACIFIC HIGHWAY

(SHEET 6 OF 6)



RAILWAY STREET / PACIFIC HIGHWAY



ANNEXURE C: EXISTING SIDRA RESULTS

(20 SHEETS)

Site: 101 [Help Street / Railway Street Existing AM]

Help Street / Railway Street Existing Conditions AM Peak Period Site Category: (None) Signals - Eixed Time Isolated - Cycle Ti

Signals - Fixed Time Isolated Cycle Time = 90 seconds (Site Optimum Cycle Time - Minimum Delay)

Move	ment P	erformanc	ce - Veh	icles								
Mov ID	Turn	Demand Total veh/h	l Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: Railwa	y Street										
1	L2	67	32.8	0.115	19.2	LOS B	2.1	18.0	0.57	0.64	0.57	44.9
2	T1	81	3.7	0.385	24.0	LOS B	5.1	37.9	0.77	0.72	0.77	41.5
3	R2	84	9.5	0.385	33.8	LOS C	5.1	37.9	0.84	0.75	0.84	38.9
Appro	ach	232	14.2	0.385	26.2	LOS B	5.1	37.9	0.74	0.71	0.74	41.4
East:	Help Stro	eet										
4	L2	184	4.9	0.271	26.5	LOS B	5.6	40.9	0.73	0.76	0.73	40.9
5	T1	197	2.0	0.271	20.9	LOS B	6.0	42.6	0.73	0.61	0.73	44.7
6	R2	68	0.0	0.220	34.0	LOS C	2.4	16.9	0.87	0.75	0.87	37.8
Appro	ach	449	2.9	0.271	25.2	LOS B	6.0	42.6	0.75	0.69	0.75	42.0
North:	Railway	/ Street										
7	L2	100	1.0	0.169	17.5	LOS B	3.7	26.8	0.56	0.62	0.56	46.8
8	T1	156	8.3	0.498	22.8	LOS B	6.4	46.7	0.75	0.70	0.75	42.2
9	R2	79	1.3	0.498	35.6	LOS C	6.4	46.7	0.87	0.76	0.87	38.3
Appro	ach	335	4.5	0.498	24.2	LOS B	6.4	46.7	0.72	0.69	0.72	42.4
West:	Help Str	reet										
10	L2	108	0.9	0.487	37.5	LOS C	8.6	61.0	0.91	0.78	0.91	37.6
11	T1	328	3.0	0.487	32.0	LOS C	8.6	61.0	0.91	0.77	0.91	38.9
12	R2	5	100.0	0.487	38.1	LOS C	8.3	61.0	0.91	0.76	0.91	37.5
Appro	ach	441	3.6	0.487	33.5	LOS C	8.6	61.0	0.91	0.77	0.91	38.6
All Ve	hicles	1457	5.3	0.498	27.6	LOS B	8.6	61.0	0.79	0.72	0.79	40.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ans						
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		
P1	South Full Crossing	53	24.3	LOS C	0.1	0.1	0.73	0.73
P2	East Full Crossing	53	19.4	LOS B	0.1	0.1	0.66	0.66
P3	North Full Crossing	53	33.9	LOS D	0.1	0.1	0.87	0.87
P4	West Full Crossing	53	17.5	LOS B	0.1	0.1	0.62	0.62
All Peo	lestrians	211	23.7	LOS C			0.72	0.72

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

V Site: 101 [Pacific Highway Site / Help Street Existing AM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment Per	formance	e - Vehi	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: F	lelp Stree	t										
5	T1	337	5.6	0.204	0.3	LOS A	0.3	2.2	0.09	0.04	0.09	56.0
6	R2	23	0.0	0.204	7.9	LOS A	0.3	2.2	0.09	0.04	0.09	55.5
Approa	ach	360	5.3	0.204	0.8	NA	0.3	2.2	0.09	0.04	0.09	55.9
North:	Site Road											
7	L2	14	0.0	0.033	6.9	LOS A	0.1	0.8	0.51	0.68	0.51	47.2
9	R2	9	0.0	0.033	11.9	LOS A	0.1	0.8	0.51	0.68	0.51	42.8
Approa	ach	23	0.0	0.033	8.9	LOS A	0.1	0.8	0.51	0.68	0.51	45.5
West:	Help Stree	et										
10	L2	36	0.0	0.037	3.3	LOS A	0.0	0.0	0.00	0.29	0.00	54.3
11	T1	377	5.6	0.183	0.0	LOS A	0.0	0.0	0.00	0.03	0.00	59.1
Approa	ach	413	5.1	0.183	0.3	NA	0.0	0.0	0.00	0.05	0.00	58.0
All Veh	icles	796	5.0	0.204	0.8	NA	0.3	2.2	0.06	0.06	0.06	55.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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✓ Site: 101 [Mcintosh Street / Help Street Site Existing AM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment Pe	rformance	e - Vehi	icles								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Site Roa	d										
1	L2	5	0.0	0.009	5.6	LOS A	0.0	0.2	0.07	0.57	0.07	46.0
3	R2	7	0.0	0.009	5.7	LOS A	0.0	0.2	0.07	0.57	0.07	49.7
Approa	ach	13	0.0	0.009	5.6	LOS A	0.0	0.2	0.07	0.57	0.07	48.3
East: F	RoadNam	е										
4	L2	38	0.0	0.029	5.5	LOS A	0.0	0.0	0.00	0.41	0.00	52.6
5	T1	17	0.0	0.029	0.0	LOS A	0.0	0.0	0.00	0.41	0.00	43.9
Approa	ach	55	0.0	0.029	3.8	NA	0.0	0.0	0.00	0.41	0.00	51.5
West:	Mcintosh	Street										
11	T1	37	0.0	0.021	0.0	LOS A	0.0	0.2	0.03	0.05	0.03	58.5
12	R2	4	0.0	0.021	2.4	LOS A	0.0	0.2	0.03	0.05	0.03	55.4
Approa	ach	41	0.0	0.021	0.3	NA	0.0	0.2	0.03	0.05	0.03	57.6
All Veh	nicles	108	0.0	0.029	2.7	NA	0.0	0.2	0.02	0.29	0.02	52.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Railway Street / Mcintosh Street Existing AM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment Pe	erformance	e - Vehi	icles								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Railway	Street										
1	L2	135	0.0	0.122	4.1	LOS A	0.0	0.0	0.00	0.34	0.00	45.2
2	T1	93	5.7	0.122	0.0	LOS A	0.0	0.0	0.00	0.34	0.00	50.2
Approa	ach	227	2.3	0.122	2.4	NA	0.0	0.0	0.00	0.34	0.00	47.1
North:	Railway	Street										
8	T1	301	7.3	0.219	0.3	LOS A	0.7	4.8	0.18	0.14	0.18	51.6
9	R2	84	0.0	0.219	6.4	LOS A	0.7	4.8	0.18	0.14	0.18	48.2
Approa	ach	385	5.7	0.219	1.6	NA	0.7	4.8	0.18	0.14	0.18	50.6
West:	Mcintosh	Street										
10	L2	12	0.0	0.025	5.8	LOS A	0.1	0.6	0.21	0.59	0.21	40.7
12	R2	13	0.0	0.025	7.9	LOS A	0.1	0.6	0.21	0.59	0.21	34.2
Approa	ach	24	0.0	0.025	6.9	LOS A	0.1	0.6	0.21	0.59	0.21	37.6
All Veh	nicles	637	4.3	0.219	2.1	NA	0.7	4.8	0.12	0.23	0.12	48.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Pacific Highway / Victoria Avenue Existing AM]

♦ Network: N101 [Existing AM]

Pacific Highway / Victoria Avenue Existing Conditions AM Peak Period Site Category: (None) Signals - Fixed Time Coordinated

Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	ement	Performa	nce -	Vehic	les									
Mov ID	Turn	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance	Prop. Queued	Effective / Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Pacifi	c Highway												
2	T1	1417	6.8	1417	6.8	0.534	6.4	LOS A	8.8	65.0	0.50	0.46	0.50	49.7
3	R2	159	5.7	159	5.7	0.614	22.7	LOS B	3.1	22.5	0.99	0.86	1.02	42.9
Appro	bach	1576	6.7	1576	6.7	0.614	8.1	LOS A	8.8	65.0	0.55	0.50	0.56	48.3
East:	Victoria	a Avenue												
4	L2	75	37.3	75	37.3	0.164	30.5	LOS C	1.5	13.7	0.76	0.74	0.76	38.6
6	R2	58	10.3	58	10.3	0.192	40.5	LOS C	1.4	10.4	0.89	0.74	0.89	26.1
Appro	bach	133	25.6	133	25.6	0.192	34.8	LOS C	1.5	13.7	0.82	0.74	0.82	34.0
North	: Pacifio	c Highway												
7	L2	55	47.3	55	47.3	0.693	16.4	LOS B	10.2	75.2	0.57	0.54	0.57	43.8
8	T1	2135	3.0	2135	3.0	0.693	13.0	LOS A	14.8	106.1	0.68	0.63	0.68	45.3
Appro	bach	2190	4.1	2190	4.1	0.693	13.1	LOS A	14.8	106.1	0.68	0.62	0.68	45.2
All Ve	hicles	3899	5.9	3899	5.9	0.693	11.8	LOS A	14.8	106.1	0.63	0.58	0.64	45.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedest	rians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94
P2	East Full Crossing	53	13.9	LOS B	0.1	0.1	0.56	0.56
P3	North Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94
All Peo	destrians	158	30.8	LOS D			0.81	0.81

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Pacific Highway / Help Street / Fullers Road Existing AM]

Pacific Highway / Help Street / Fullers Road Existing Conditions AM Peak Period Site Category: (None) Signals - Fixed Time Coordinated Cycle Time =

Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	ement	Performa	ince -	Vehic	les									
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	Aver. No.A	verage
U		Iotal	ΗV	Iotal	HV	Sath	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Pacifi	c Highway												
1	L2	331	5.4	331	5.4	0.861	29.0	LOS C	11.5	84.6	0.91	0.96	1.25	34.9
2	T1	1098	7.7	1098	7.7	0.861	27.7	LOS B	12.3	91.5	0.94	0.92	1.09	16.2
Appro	bach	1429	7.2	1429	7.2	0.861	28.0	LOS B	12.3	91.5	0.93	0.93	1.13	22.8
East:	Help S	treet												
4	L2	23	21.7	23	21.7	0.080	37.2	LOS C	0.5	4.4	0.86	0.70	0.86	4.5
5	T1	297	4.4	297	4.4	0.931	58.1	LOS E	10.1	73.4	1.00	1.14	1.54	21.9
Appro	bach	320	5.6	320	5.6	0.931	56.6	LOS E	10.1	73.4	0.99	1.11	1.49	21.4
North	: Pacifi	c Highway												
7	L2	9	11.1	9	11.1	0.877	38.2	LOS C	7.7	55.0	1.00	1.00	1.10	7.6
8	T1	1854	3.0	1854	3.0	0.877	35.5	LOS C	7.7	55.0	1.00	1.00	1.10	6.7
9	R2	400	3.5	400	3.5	0.876	40.5	LOS C	7.6	55.0	1.00	1.02	1.41	27.3
Appro	ach	2263	3.1	2263	3.1	0.877	36.4	LOS C	7.7	55.0	1.00	1.00	1.15	12.7
West:	Fullers	Road												
10	L2	354	7.3	354	7.3	0.550	26.1	LOS B	8.3	61.7	0.80	0.80	0.80	33.1
11	T1	383	5.0	383	5.0	0.550	26.8	LOS B	8.3	61.7	0.87	0.76	0.87	32.9
12	R2	292	9.6	292	9.6	0.680	50.1	LOS D	4.2	32.2	1.00	0.94	1.47	23.0
Appro	bach	1029	7.1	1029	7.1	0.680	33.2	LOS C	8.3	61.7	0.88	0.82	1.02	29.4
All Ve	hicles	5041	5.2	5041	5.2	0.931	34.6	LOS C	12.3	91.5	0.96	0.95	1.14	20.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ins						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	32	39.3	LOS D	0.1	0.1	0.93	0.93
P2	East Full Crossing	32	15.0	LOS B	0.0	0.0	0.58	0.58
P3	North Full Crossing	32	34.7	LOS D	0.1	0.1	0.88	0.88
P4	West Full Crossing	32	33.0	LOS D	0.1	0.1	0.86	0.86
All Peo	lestrians	126	30.5	LOS D			0.81	0.81

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

V Site: 101 [Pacific Highway / Mcintosh Street Existing AM]

♦ Network: N101 [Existing AM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	nce -	Vehic	les									
Mov ID	Turn	Demand F Total	lows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Bacł Vehicles	c of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Pacifi	ic Highway												
3a	R1	1540	9.3	1540	9.3	0.347	3.0	LOS A	0.0	0.0	0.00	0.45	0.00	35.6
Appro	bach	1540	9.3	1540	9.3	0.347	3.0	NA	0.0	0.0	0.00	0.45	0.00	35.6
East:	Mcinto	sh Street												
4	L2	17	0.0	17	0.0	0.058	7.5	LOS A	0.0	0.3	0.59	0.74	0.59	14.2
Appro	bach	17	0.0	17	0.0	0.058	7.5	LOS A	0.0	0.3	0.59	0.74	0.59	14.2
North	East: P	acific Highw	vay											
24b	L3	37	0.0	37	0.0	0.435	6.5	LOS A	20.2	144.6	0.00	0.51	0.00	43.2
24a	L1	2408	2.7	2408	2.7	0.435	4.5	LOS A	20.7	148.3	0.00	0.50	0.00	45.4
Appro	bach	2445	2.6	2445	2.6	0.435	4.5	NA	20.7	148.3	0.00	0.50	0.00	45.3
All Ve	hicles	4002	5.2	4002	5.2	0.435	3.9	NA	20.7	148.3	0.00	0.48	0.00	43.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 101 [Pacific Highway / Railway Street Existing AM]

♦ Network: N101 [Existing AM]

New Site

Site Category: (None) Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	ovement Performance - Vehicles													
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	Aver. No.A	verage
ט ו		Iotai	ΗV	Iotal	ΗV	Sath	Delay	Service	venicies	Distance	Queuea	Stop Rate	Cycles a	speea
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Railw	ay Street												
1b	L3	12	0.0	12	0.0	0.374	53.3	LOS D	1.2	8.5	0.99	0.73	0.99	9.4
3a	R1	76	6.9	76	6.9	0.374	51.3	LOS D	1.3	9.4	0.99	0.74	0.99	25.5
Appro	ach	87	6.0	87	6.0	0.374	51.5	LOS D	1.3	9.4	0.99	0.74	0.99	24.0
NorthEast: Pa		acific High	way											
24a	L1	206	1.5	206	1.5	0.833	21.9	LOS B	19.8	141.5	0.81	0.82	0.87	39.6
25	T1	2536	2.5	2536	2.5	0.833	17.3	LOS B	22.5	160.8	0.83	0.82	0.89	38.1
Appro	ach	2742	2.4	2742	2.4	0.833	17.6	LOS B	22.5	160.8	0.83	0.82	0.89	38.2
South	West: I	Pacific High	hway											
31	T1	1534	8.5	1534	8.5	0.485	0.7	LOS A	3.0	22.2	0.10	0.09	0.10	59.1
32b	R3	76	22.2	76	22.2	0.249	27.8	LOS B	1.7	14.1	0.84	0.79	0.84	25.5
Appro	ach	1609	9.2	1609	9.2	0.485	2.0	LOS A	3.0	22.2	0.14	0.13	0.14	57.2
All Ve	hicles	4439	4.9	4439	4.9	0.833	12.6	LOS A	22.5	160.8	0.58	0.57	0.62	44.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians														
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94						
P8	SouthWest Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94						
All Pe	destrians	105	39.3	LOS D			0.94	0.94						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Help Street / Railway Street Existing PM]

Help Street / Railway Street Existing Conditions AM Peak Period Site Category: (None) Signals - Eixed Time Isolated Cycle Tir

Signals - Fixed Time Isolated Cycle Time = 90 seconds (Site Optimum Cycle Time - Minimum Delay)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	l Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South	: Railway	Street												
1	L2	102	27.5	0.136	16.1	LOS B	2.5	21.2	0.51	0.66	0.51	46.4		
2	T1	110	4.5	0.453	21.5	LOS B	7.6	55.9	0.76	0.74	0.76	42.6		
3	R2	133	6.0	0.453	29.3	LOS C	7.6	55.9	0.81	0.75	0.81	40.8		
Appro	ach	345	11.9	0.453	22.9	LOS B	7.6	55.9	0.70	0.72	0.70	42.9		
East: I	Help Stree	et												
4	L2	95	12.6	0.179	30.2	LOS C	3.1	23.7	0.77	0.74	0.77	39.2		
5	T1	280	3.6	0.472	27.2	LOS B	10.0	72.3	0.86	0.73	0.86	41.5		
6	R2	57	1.8	0.209	37.4	LOS C	2.1	15.1	0.90	0.74	0.90	36.5		
Approach		432	5.3	0.472	29.2	LOS C	10.0	72.3	0.84	0.73	0.84	40.2		
North:	Railway S	Street												
7	L2	106	2.8	0.188	14.7	LOS B	4.0	29.5	0.49	0.57	0.49	48.8		
8	T1	113	11.5	0.553	12.0	LOS A	6.5	48.5	0.55	0.61	0.55	47.9		
9	R2	165	7.9	0.553	30.8	LOS C	6.5	48.5	0.82	0.78	0.82	39.2		
Appro	ach	384	7.6	0.553	20.8	LOS B	6.5	48.5	0.65	0.67	0.65	43.9		
West:	Help Stre	et												
10	L2	46	2.2	0.505	42.6	LOS D	7.0	49.6	0.95	0.78	0.95	36.1		
11	T1	280	1.8	0.505	37.1	LOS C	7.0	49.6	0.95	0.78	0.95	37.0		
12	R2	5	100.0	0.505	43.3	LOS D	6.6	47.9	0.95	0.77	0.95	35.6		
Appro	ach	331	3.3	0.505	38.1	LOS C	7.0	49.6	0.95	0.78	0.95	36.9		
All Vel	nicles	1492	7.0	0.553	27.5	LOS B	10.0	72.3	0.79	0.72	0.79	40.9		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians													
Mov	-	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m							
P1	South Full Crossing	53	28.9	LOS C	0.1	0.1	0.80	0.80					
P2	East Full Crossing	53	15.6	LOS B	0.1	0.1	0.59	0.59					
P3	North Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P4	West Full Crossing	53	13.9	LOS B	0.1	0.1	0.56	0.56					
All Peo	All Pedestrians		24.4	LOS C			0.72	0.72					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

✓ Site: 101 [Pacific Highway Site / Help Street Existing PM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
East: H	Help Stree	et												
5	T1	539	6.4	0.296	0.1	LOS A	0.1	0.8	0.02	0.01	0.02	59.0		
6	R2	8	0.0	0.296	7.8	LOS A	0.1	0.8	0.02	0.01	0.02	56.3		
Approa	ach	547	6.3	0.296	0.2	NA	0.1	0.8	0.02	0.01	0.02	58.9		
North: Site Road		ł												
7	L2	31	0.0	0.084	6.7	LOS A	0.3	2.1	0.53	0.71	0.53	46.2		
9	R2	20	0.0	0.084	15.1	LOS B	0.3	2.1	0.53	0.71	0.53	41.8		
Approa	ach	51	0.0	0.084	10.0	LOS A	0.3	2.1	0.53	0.71	0.53	44.5		
West:	Help Stree	et												
10	L2	13	0.0	0.031	3.3	LOS A	0.0	0.0	0.00	0.12	0.00	55.9		
11	T1	344	2.8	0.155	0.0	LOS A	0.0	0.0	0.00	0.02	0.00	59.4		
Approa	ach	357	2.7	0.155	0.1	NA	0.0	0.0	0.00	0.02	0.00	59.1		
All Veh	nicles	955	4.6	0.296	0.7	NA	0.3	2.1	0.04	0.05	0.04	56.2		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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✓ Site: 101 [Mcintosh Street / Help Street Site Existing PM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South:	Site Roa	ıd												
1	L2	12	0.0	0.032	5.8	LOS A	0.1	0.7	0.20	0.56	0.20	45.5		
3	R2	27	0.0	0.032	5.9	LOS A	0.1	0.7	0.20	0.56	0.20	49.2		
Approa	ach	39	0.0	0.032	5.9	LOS A	0.1	0.7	0.20	0.56	0.20	48.2		
East: RoadName		ne												
4	L2	5	0.0	0.056	5.5	LOS A	0.0	0.0	0.00	0.03	0.00	57.1		
5	T1	104	0.0	0.056	0.0	LOS A	0.0	0.0	0.00	0.03	0.00	58.5		
Approa	ach	109	0.0	0.056	0.3	NA	0.0	0.0	0.00	0.03	0.00	58.2		
West:	Mcintosh	Street												
11	T1	31	0.0	0.016	0.0	LOS A	0.0	0.0	0.02	0.02	0.02	59.4		
12	R2	1	0.0	0.016	2.5	LOS A	0.0	0.0	0.02	0.02	0.02	55.6		
Approa	ach	32	0.0	0.016	0.1	NA	0.0	0.0	0.02	0.02	0.02	59.0		
All Veh	nicles	180	0.0	0.056	1.5	NA	0.1	0.7	0.05	0.14	0.05	53.1		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Railway Street / Mcintosh Street Existing PM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South:	Railway S	Street												
1	L2	34	0.0	0.138	4.1	LOS A	0.0	0.0	0.00	0.07	0.00	51.0		
2	T1	233	1.4	0.138	0.0	LOS A	0.0	0.0	0.00	0.07	0.00	57.6		
Approa	ach	266	1.2	0.138	0.5	NA	0.0	0.0	0.00	0.07	0.00	56.6		
North:	North: Railway Str													
8	T1	296	7.5	0.177	0.1	LOS A	0.2	1.6	0.08	0.05	0.08	56.5		
9	R2	25	0.0	0.177	6.5	LOS A	0.2	1.6	0.08	0.05	0.08	51.0		
Approa	ach	321	6.9	0.177	0.6	NA	0.2	1.6	0.08	0.05	0.08	55.8		
West: I	Mcintosh \$	Street												
10	L2	84	0.0	0.176	6.4	LOS A	0.7	4.6	0.39	0.67	0.39	40.0		
12	R2	83	0.0	0.176	8.5	LOS A	0.7	4.6	0.39	0.67	0.39	33.4		
Approa	ach	167	0.0	0.176	7.4	LOS A	0.7	4.6	0.39	0.67	0.39	37.0		
All Veh	icles	755	3.3	0.177	2.1	NA	0.7	4.6	0.12	0.20	0.12	49.6		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Pacific Highway / Victoria Avenue Existing PM]

♦ Network: N101 [Existing PM]

Pacific Highway / Victoria Avenue Existing Conditions AM Peak Period Site Category: (None) Signals - Fixed Time Coordinated

Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	lovement Performance - Vehicles													
Mov ID	Turn	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	i: Pacifi	c Highway												
2	T1	1958	3.7	1958	3.7	0.496	6.2	LOS A	8.0	57.5	0.48	0.44	0.48	50.0
3	R2	136	6.6	136	6.6	0.439	15.5	LOS B	1.5	11.3	0.73	0.76	0.73	46.6
Appro	bach	2094	3.9	2094	3.9	0.496	6.8	LOS A	8.0	57.5	0.50	0.46	0.50	49.6
East: Victoria		a Avenue												
4	L2	89	32.6	89	32.6	0.190	30.6	LOS C	1.8	15.9	0.77	0.75	0.77	38.6
6	R2	79	10.1	79	10.1	0.280	41.2	LOS C	1.9	14.5	0.91	0.76	0.91	25.8
Appro	bach	168	22.0	168	22.0	0.280	35.6	LOS C	1.9	15.9	0.83	0.75	0.83	33.4
North	: Pacifi	c Highway												
7	L2	63	36.5	63	36.5	0.651	13.0	LOS A	7.0	51.8	0.42	0.41	0.42	47.1
8	T1	1296	3.8	1296	3.8	0.651	9.0	LOS A	9.7	70.3	0.50	0.47	0.50	48.8
Appro	bach	1359	5.3	1359	5.3	0.651	9.2	LOS A	9.7	70.3	0.50	0.46	0.50	48.7
All Ve	hicles	3621	5.3	3621	5.3	0.651	9.0	LOS A	9.7	70.3	0.51	0.47	0.51	47.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Bacl Pedestrian ped	k of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P2	East Full Crossing	53	13.9	LOS B	0.1	0.1	0.56	0.56					
P3	North Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
All Peo	All Pedestrians		30.8	LOS D			0.81	0.81					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Pacific Highway / Help Street / Fullers Road Existing PM]

Pacific Highway / Help Street / Fullers Road Existing Conditions PM Peak Period Site Category: (None) Signals - Fixed Time Coordinated Cycle Time =

Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	ovement Performance - Vehicles													
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	Aver. No.A	verage
ID		Iotal	ΗV	Iotal	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Pacifi	c Highway												
1	L2	440	2.3	440	2.3	0.760	21.0	LOS B	11.2	80.6	0.74	0.83	0.99	39.5
2	T1	1513	4.7	1513	4.7	0.760	17.9	LOS B	13.1	95.2	0.77	0.72	0.82	21.7
Appro	ach	1953	4.1	1953	4.1	0.760	18.6	LOS B	13.1	95.2	0.76	0.75	0.85	28.5
East:	Help S	treet												
4	L2	50	16.0	50	16.0	0.169	37.9	LOS C	1.2	9.3	0.88	0.73	0.88	4.5
5	T1	462	5.4	462	5.4	0.730	40.3	LOS C	6.8	47.9	0.99	0.88	1.10	27.2
Appro	bach	512	6.4	512	6.4	0.730	40.1	LOS C	6.8	47.9	0.98	0.87	1.08	25.9
North	: Pacifi	c Highway												
7	L2	16	0.0	16	0.0	0.516	16.3	LOS B	7.6	55.0	0.64	0.58	0.64	13.1
8	T1	1096	3.8	1096	3.8	0.516	12.5	LOS A	7.6	55.0	0.63	0.57	0.63	15.8
9	R2	163	9.8	163	9.8	0.833	20.3	LOS B	2.4	18.2	0.90	0.83	1.01	36.7
Appro	ach	1275	4.5	1275	4.5	0.833	13.5	LOS A	7.6	55.0	0.66	0.60	0.68	22.5
West:	Fullers	Road												
10	L2	390	4.1	390	4.1	0.524	25.8	LOS B	7.9	57.4	0.78	0.81	0.78	32.9
11	T1	323	2.8	323	2.8	0.524	27.5	LOS B	7.9	57.4	0.87	0.75	0.87	32.9
12	R2	154	16.2	154	16.2	0.623	46.2	LOS D	4.0	32.2	0.98	0.88	1.24	24.2
Appro	bach	867	5.8	867	5.8	0.623	30.0	LOS C	7.9	57.4	0.85	0.80	0.90	30.9
All Ve	hicles	4607	4.8	4607	4.8	0.833	21.7	LOS B	13.1	95.2	0.78	0.73	0.84	27.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	lovement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P2	East Full Crossing	53	16.2	LOS B	0.1	0.1	0.60	0.60					
P3	North Full Crossing	53	34.7	LOS D	0.1	0.1	0.88	0.88					
P4	West Full Crossing	53	22.1	LOS C	0.1	0.1	0.70	0.70					
All Peo	lestrians	211	28.1	LOS C			0.78	0.78					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

V Site: 101 [Pacific Highway / Mcintosh Street Existing PM]

♦ Network: N101 [Existing PM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand F Total	lows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Pacifi	c Highway												
3a	R1	2012	3.1	2012	3.1	0.459	3.1	LOS A	0.0	0.0	0.00	0.45	0.00	35.6
Appro	ach	2012	3.1	2012	3.1	0.459	3.1	NA	0.0	0.0	0.00	0.45	0.00	35.6
East:	Mcinto	sh Street												
4	L2	104	0.0	104	0.0	0.253	5.4	LOS A	0.2	1.4	0.50	0.70	0.50	18.1
Appro	ach	104	0.0	104	0.0	0.253	5.4	LOS A	0.2	1.4	0.50	0.70	0.50	18.1
North	East: P	acific Highv	vay											
24b	L3	31	0.0	31	0.0	0.284	6.5	LOS A	1.0	7.2	0.00	0.51	0.00	43.1
24a	L1	1552	3.7	1552	3.7	0.284	4.5	LOS A	1.0	7.2	0.00	0.51	0.00	45.4
Appro	ach	1582	3.7	1582	3.7	0.284	4.5	NA	1.0	7.2	0.00	0.51	0.00	45.3
All Ve	hicles	3698	3.2	3698	3.2	0.459	3.7	NA	1.0	7.2	0.01	0.48	0.01	41.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 101 [Pacific Highway / Railway Street Existing PM]

♦ Network: N101 [Existing PM]

New Site

Site Category: (None) Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Movement Performance - Vehicles														
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	Aver. No.A	verage
ID		Total	ΗV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Tale		km/h
South: Railway Street														
1b	L3	20	0.0	20	0.0	0.372	41.6	LOS C	3.1	22.1	0.92	0.78	0.92	11.5
3a	R1	243	1.3	243	1.3	0.372	39.7	LOS C	3.2	22.7	0.92	0.78	0.92	29.3
Appro	ach	263	1.2	263	1.2	0.372	39.8	LOS C	3.2	22.7	0.92	0.78	0.92	28.4
NorthEast: Pacific Highway														
24a	L1	248	3.8	248	3.8	0.614	18.6	LOS B	11.4	82.2	0.70	0.70	0.70	41.2
25	T1	1708	3.3	1708	3.3	0.614	14.2	LOS A	11.7	84.6	0.72	0.67	0.72	40.5
Appro	ach	1957	3.4	1957	3.4	0.614	14.8	LOS B	11.7	84.6	0.72	0.67	0.72	40.7
South	SouthWest: Pacific Highway													
31	T1	2191	2.0	2191	2.0	0.813	6.7	LOS A	20.3	144.2	0.55	0.52	0.55	52.0
32b	R3	53	32.0	53	32.0	0.261	12.4	LOS A	0.4	3.1	0.26	0.66	0.26	35.2
Appro	ach	2243	2.7	2243	2.7	0.813	6.8	LOS A	20.3	144.2	0.54	0.52	0.55	51.7
All Ve	hicles	4463	2.9	4463	2.9	0.813	12.3	LOS A	20.3	144.2	0.64	0.60	0.64	45.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians											
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate			
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94			
P8	SouthWest Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94			
All Pe	destrians	105	39.3	LOS D			0.94	0.94			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



ANNEXURE D: SWEPT PATH ANALYSIS

(3 SHEETS)







ANNEXURE E: JOURNEY TO WORK DATA

(SHEET 1 OF 1)

Sum of EMPLOYED_PERSONS	Column Labels	
Row Labels	2011	2016
Bicycle	0.67%	0.53%
Bus	5.48%	6.35%
Car as driver	51.21%	43.67%
Car as passenger	4.26%	3.38%
Motorbike	0.68%	0.71%
Other mode	0.29%	0.33%
Taxi	0.32%	0.20%
Train	29.39%	33.83%
Tram	0.06%	0.02%
Truck	0.26%	0.20%
Walked only	7.39%	7.47%
Grand Total	1	

2011 VS 2016 JOURNEY TO WORK DATA FOR CHATSWOOD (EAST) – ARTARMON SA2 AREA



ANNEXURE F: TRAFFIC DISTRIBUTION

(1 SHEET)





ANNEXURE G: FUTURE SIDRA RESULTS – PROPOSED ACCESS

(18 SHEETS)

V Site: 101 [Mcintosh Street / Help Street Site Future AM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand l Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Site Roa	d										
1	L2	43	0.0	0.134	5.6	LOS A	0.5	3.4	0.06	0.59	0.06	45.1
3	R2	99	0.0	0.134	7.0	LOS A	0.5	3.4	0.06	0.59	0.06	48.9
Approa	ach	142	0.0	0.134	6.6	LOS A	0.5	3.4	0.06	0.59	0.06	47.8
East: F	RoadNam	е										
4	L2	512	0.0	0.282	5.5	LOS A	0.0	0.0	0.00	0.56	0.00	50.8
5	T1	13	0.0	0.282	0.0	LOS A	0.0	0.0	0.00	0.56	0.00	39.6
Approach		524	0.0	0.282	5.4	NA	0.0	0.0	0.00	0.56	0.00	50.7
West: Mcintosh Street												
11	T1	37	0.0	0.074	1.9	LOS A	0.4	2.5	0.50	0.40	0.50	45.0
12	R2	57	0.0	0.074	4.4	LOS A	0.4	2.5	0.50	0.40	0.50	50.7
Approa	ach	94	0.0	0.074	3.4	NA	0.4	2.5	0.50	0.40	0.50	49.6
All Veh	nicles	760	0.0	0.282	5.4	NA	0.5	3.4	0.07	0.55	0.07	50.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Railway Street / Mcintosh Street Future AM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South:	Railway	Street												
1	L2	443	0.0	0.285	4.1	LOS A	0.0	0.0	0.00	0.47	0.00	42.8		
2	T1	87	6.0	0.285	0.0	LOS A	0.0	0.0	0.00	0.47	0.00	47.2		
Approa	ach	531	1.0	0.285	3.4	NA	0.0	0.0	0.00	0.47	0.00	43.5		
North: Railway		Street												
8	T1	301	7.3	0.394	2.7	LOS A	3.0	21.4	0.57	0.39	0.72	41.5		
9	R2	241	0.0	0.394	9.0	LOS A	3.0	21.4	0.57	0.39	0.72	41.5		
Approa	ach	542	4.1	0.394	5.5	NA	3.0	21.4	0.57	0.39	0.72	41.5		
West:	Mcintosh	Street												
10	L2	79	0.0	0.130	5.8	LOS A	0.5	3.3	0.20	0.59	0.20	39.8		
12	R2	40	0.0	0.130	11.0	LOS A	0.5	3.3	0.20	0.59	0.20	33.3		
Approa	ach	119	0.0	0.130	7.6	LOS A	0.5	3.3	0.20	0.59	0.20	37.8		
All Veh	nicles	1192	2.3	0.394	4.8	NA	3.0	21.4	0.28	0.45	0.35	41.8		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: C:\Users\McLar\Documents\20 09 08 - SIDRA for Chatswood.sip8

Site: 101 [Help Street / Railway Street Future AM]

Help Street / Railway Street Existing Conditions AM Peak Period Site Category: (None) Signals - Eixed Time Isolated Cycle

Signals - Fixed Time Isolated Cycle Time = 90 seconds (Site Optimum Cycle Time - Minimum Delay)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	l Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South	: Railway	Street												
1	L2	49	44.9	0.219	16.8	LOS B	4.8	36.5	0.54	0.52	0.54	47.7		
2	T1	230	1.3	0.732	19.8	LOS B	6.8	49.8	0.66	0.62	0.71	44.5		
3	R2	84	9.5	0.732	49.9	LOS D	6.8	49.8	1.00	0.89	1.18	33.2		
Appro	ach	363	9.1	0.732	26.4	LOS B	6.8	49.8	0.72	0.67	0.80	41.6		
East:	Help Stre	et												
4	L2	184	4.9	0.330	31.4	LOS C	6.3	45.6	0.81	0.78	0.81	38.8		
5	T1	191	2.1	0.319	25.7	LOS B	6.4	46.0	0.81	0.67	0.81	42.2		
6	R2	167	0.0	0.721	47.5	LOS D	7.3	50.9	1.00	0.93	1.36	33.2		
Approach		542	2.4	0.721	34.3	LOS C	7.3	50.9	0.87	0.79	0.98	37.9		
North:	Railway	Street												
7	L2	113	0.9	0.280	15.4	LOS B	5.1	37.1	0.52	0.57	0.52	48.6		
8	T1	156	8.3	0.824	16.8	LOS B	5.8	41.9	0.60	0.64	0.67	45.3		
9	R2	92	1.1	0.824	55.1	LOS D	5.8	41.9	1.00	0.96	1.40	31.4		
Appro	ach	361	4.2	0.824	26.1	LOS B	5.8	41.9	0.68	0.70	0.81	41.5		
West:	Help Stre	eet												
10	L2	148	0.7	0.833	50.6	LOS D	11.4	80.8	1.00	0.98	1.28	32.9		
11	T1	318	3.1	0.833	45.2	LOS D	11.4	80.8	1.00	0.98	1.28	34.1		
12	R2	7	100.0	0.833	51.4	LOS D	11.0	81.2	1.00	0.98	1.29	33.0		
Appro	ach	473	3.8	0.833	47.0	LOS D	11.4	81.2	1.00	0.98	1.28	33.7		
All Ve	hicles	1739	4.5	0.833	34.4	LOS C	11.4	81.2	0.83	0.80	0.99	38.0		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov	-	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m								
P1	South Full Crossing	53	28.9	LOS C	0.1	0.1	0.80	0.80						
P2	East Full Crossing	53	15.6	LOS B	0.1	0.1	0.59	0.59						
P3	North Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94						
P4	West Full Crossing	53	13.9	LOS B	0.1	0.1	0.56	0.56						
All Peo	All Pedestrians		24.4	LOS C			0.72	0.72						

Site: 101 [Pacific Highway / Victoria Avenue Future AM]

Pacific Highway / Victoria Avenue Existing Conditions AM Peak Period Site Category: (None) Signals - Eixed Time Coordinated

Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance	Prop. Queued	Effective / Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	i: Pacifi	c Highway												
2	T1	1417	6.8	1417	6.8	0.523	5.1	LOS A	8.1	59.9	0.45	0.41	0.45	40.1
3	R2	289	3.1	289	3.1	0.895	58.1	LOS E	8.2	59.2	1.00	1.14	1.74	24.2
Appro	bach	1706	6.2	1706	6.2	0.895	14.1	LOS A	8.2	59.9	0.55	0.54	0.67	31.1
East:	Victoria	a Avenue												
4	L2	75	37.3	75	37.3	0.164	30.5	LOS C	1.5	13.7	0.76	0.74	0.76	32.1
6	R2	58	10.3	58	10.3	0.249	43.9	LOS D	1.4	11.0	0.93	0.75	0.93	24.9
Appro	bach	133	25.6	133	25.6	0.249	36.3	LOS C	1.5	13.7	0.83	0.74	0.83	29.0
North	: Pacifi	c Highway												
7	L2	55	47.3	55	47.3	0.704	16.5	LOS B	10.5	77.7	0.58	0.55	0.58	43.7
8	T1	2169	2.9	2169	2.9	0.704	13.2	LOS A	15.1	108.3	0.69	0.63	0.69	35.8
Appro	bach	2224	4.0	2224	4.0	0.704	13.2	LOS A	15.1	108.3	0.69	0.63	0.69	36.1
All Ve	hicles	4063	5.6	4063	5.6	0.895	14.4	LOS A	15.1	108.3	0.63	0.60	0.69	33.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians														
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate							
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94							
P2	East Full Crossing	53	13.9	LOS B	0.1	0.1	0.56	0.56							
P3	North Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94							
All Peo	All Pedestrians		30.8	LOS D			0.81	0.81							

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Pacific Highway / Help Street / Fullers Road Future + Network: N101 [Future AM] AM]

Pacific Highway / Help Street / Fullers Road Existing Conditions AM Peak Period Site Category: (None) Signals - Fixed Time Coordinated Cycle Time =

Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	ovement Performance - Vehicles													
Mov	Turn	Demand Total	Flows	Arrival Total	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Back	of Queue	Prop.	Effective /	Aver. No.A	verage
		iotai	110	Total	110	Call	Delay		Venieles	Distance	Queueu	Rate	0901030	opeed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	i: Pacif	ic Highway	/											
1	L2	331	5.4	331	5.4	0.861	36.5	LOS C	12.2	90.0	0.95	0.99	1.31	31.5
2	T1	1098	7.7	1098	7.7	0.861	35.0	LOS C	13.0	96.8	0.97	0.97	1.17	13.6
Appro	bach	1429	7.2	1429	7.2	0.861	35.4	LOS C	13.0	96.8	0.96	0.98	1.20	19.6
East:	Help S	Street												
4	L2	23	21.7	23	21.7	0.080	37.2	LOS C	0.5	4.4	0.86	0.70	0.86	4.5
5	T1	310	4.2	310	4.2	0.970	70.1	LOS E	11.7	84.8	1.00	1.24	1.70	19.4
Appro	bach	333	5.4	333	5.4	0.970	67.8	LOS E	11.7	84.8	0.99	1.20	1.65	19.0
North	: Pacifi	ic Highway												
7	L2	1	100.0	1	100. 0	0.890	40.0	LOS C	7.7	55.0	1.00	1.01	1.12	7.3
8	T1	1890	2.9	1890	2.9	0.890	37.6	LOS C	7.7	55.0	1.00	1.01	1.12	6.4
9	R2	398	3.5	398	3.5	0.872	40.0	LOS C	7.6	55.0	1.00	1.02	1.40	27.5
Appro	bach	2289	3.1	2289	3.1	0.890	38.0	LOS C	7.7	55.0	1.00	1.01	1.17	12.2
West	Fuller	s Road												
10	L2	354	7.3	354	7.3	0.594	28.0	LOS B	9.1	67.1	0.83	0.81	0.83	32.2
11	T1	426	4.5	426	4.5	0.594	27.4	LOS B	9.1	67.1	0.89	0.78	0.89	32.5
12	R2	292	9.6	292	9.6	0.709	51.4	LOS D	4.3	32.8	1.00	0.95	1.56	22.6
Appro	bach	1072	6.8	1072	6.8	0.709	34.1	LOS C	9.1	67.1	0.90	0.84	1.05	29.0
All Ve	hicles	5123	5.2	5123	5.2	0.970	38.4	LOS C	13.0	96.8	0.97	0.98	1.18	19.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

 $\mathsf{HV}\xspace(\%)$ values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	lovement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94						
P2	East Full Crossing	53	15.1	LOS B	0.1	0.1	0.58	0.58						
P3	North Full Crossing	53	34.7	LOS D	0.1	0.1	0.88	0.88						
P4	West Full Crossing	53	33.0	LOS D	0.1	0.1	0.86	0.86						
All Peo	lestrians	211	30.5	LOS D			0.81	0.81						

V Site: 101 [Pacific Highway / Mcintosh Street Future AM]

♦ Network: N101 [Future AM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	nce -	Vehic	les									
Mov ID	Turn	Demand F Total	lows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Pacifi	c Highway												
3a	R1	1540	9.3	1540	9.3	0.347	3.0	LOS A	0.0	0.0	0.00	0.45	0.00	35.6
Appro	ach	1540	9.3	1540	9.3	0.347	3.0	NA	0.0	0.0	0.00	0.45	0.00	35.6
East: Mcintosh Street														
4	L2	53	0.0	53	0.0	0.167	7.2	LOS A	0.1	0.9	0.58	0.77	0.58	14.6
Appro	ach	53	0.0	53	0.0	0.167	7.2	LOS A	0.1	0.9	0.58	0.77	0.58	14.6
North	East: P	acific Highw	vay											
24b	L3	89	0.0	89	0.0	0.440	6.5	LOS A	20.8	148.3	0.00	0.52	0.00	42.9
24a	L1	2376	2.7	2376	2.7	0.440	4.5	LOS A	22.3	160.0	0.00	0.51	0.00	45.3
Appro	ach	2465	2.6	2465	2.6	0.440	4.5	NA	22.3	160.0	0.00	0.51	0.00	45.2
All Ve	hicles	4058	5.1	4058	5.1	0.440	4.0	NA	22.3	160.0	0.01	0.49	0.01	43.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 101 [Pacific Highway / Railway Street Future AM]

♦ Network: N101 [Future AM]

New Site

Site Category: (None) Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	lovement Performance - Vehicles													
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	ver. No.A	verage
U		Iotal	ΗV	Iotal	HV	Sath	Delay	Service	Venicles	Distance	Queued	Stop Rate	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Railw	ay Street												
1b	L3	12	0.0	12	0.0	0.619	54.8	LOS D	2.1	15.0	1.00	0.80	1.11	9.1
3a	R1	138	3.8	138	3.8	0.619	52.8	LOS D	2.2	15.9	1.00	0.80	1.10	25.2
Appro	ach	149	3.5	149	3.5	0.619	52.9	LOS D	2.2	15.9	1.00	0.80	1.10	24.3
NorthEast: Pa		acific High	way											
24a	L1	363	0.9	363	0.9	0.892	30.5	LOS C	26.2	186.6	0.86	0.93	1.02	34.2
25	T1	2503	2.5	2503	2.5	0.892	26.7	LOS B	29.8	213.1	0.90	0.97	1.06	31.8
Appro	ach	2866	2.3	2866	2.3	0.892	27.2	LOS B	29.8	213.1	0.90	0.96	1.05	32.2
South	West:	Pacific High	nway											
31	T1	1534	8.5	1534	8.5	0.485	0.7	LOS A	3.0	22.2	0.10	0.09	0.10	59.1
32b	R3	76	22.2	76	22.2	0.250	38.7	LOS C	2.1	17.3	1.00	0.81	1.00	21.2
Appro	ach	1609	9.2	1609	9.2	0.485	2.5	LOS A	3.0	22.2	0.14	0.13	0.14	56.5
All Ve	hicles	4625	4.7	4625	4.7	0.892	19.4	LOS B	29.8	213.1	0.64	0.67	0.74	38.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94						
P8	SouthWest Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94						
All Pe	destrians	105	39.3	LOS D			0.94	0.94						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

V Site: 101 [Mcintosh Street / Help Street Site Future PM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South:	Site Roa	d												
1	L2	143	0.0	0.400	5.9	LOS A	1.9	13.4	0.29	0.60	0.29	45.1		
3	R2	334	0.0	0.400	6.5	LOS A	1.9	13.4	0.29	0.60	0.29	48.9		
Approa	ach	477	0.0	0.400	6.3	LOS A	1.9	13.4	0.29	0.60	0.29	47.9		
East: RoadName		e												
4	L2	116	0.0	0.109	5.5	LOS A	0.0	0.0	0.00	0.33	0.00	53.5		
5	T1	91	0.0	0.109	0.0	LOS A	0.0	0.0	0.00	0.33	0.00	46.2		
Approa	ach	206	0.0	0.109	3.1	NA	0.0	0.0	0.00	0.33	0.00	52.1		
West:	Mcintosh	Street												
11	T1	31	0.0	0.024	0.3	LOS A	0.1	0.5	0.19	0.15	0.19	54.5		
12	R2	13	0.0	0.024	2.8	LOS A	0.1	0.5	0.19	0.15	0.19	54.2		
Approa	ach	43	0.0	0.024	1.0	NA	0.1	0.5	0.19	0.15	0.19	54.3		
All Veh	nicles	726	0.0	0.400	5.1	NA	1.9	13.4	0.20	0.50	0.20	48.9		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: C:\Users\McLar\Documents\20 09 08 - SIDRA for Chatswood.sip8

V Site: 101 [Railway Street / Mcintosh Street Future PM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand l Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South:	Railway	Street												
1	L2	94	0.0	0.162	4.1	LOS A	0.0	0.0	0.00	0.18	0.00	48.7		
2	T1	216	1.5	0.162	0.0	LOS A	0.0	0.0	0.00	0.18	0.00	54.6		
Approa	ach	309	1.0	0.162	1.2	NA	0.0	0.0	0.00	0.18	0.00	52.6		
North: Railway		Street												
8	T1	296	7.5	0.205	0.3	LOS A	0.5	3.9	0.18	0.11	0.18	52.6		
9	R2	62	0.0	0.205	6.7	LOS A	0.5	3.9	0.18	0.11	0.18	48.8		
Approa	ach	358	6.2	0.205	1.5	NA	0.5	3.9	0.18	0.11	0.18	51.7		
West:	Mcintosh	Street												
10	L2	303	0.0	0.470	7.3	LOS A	3.1	21.4	0.44	0.73	0.58	38.4		
12	R2	171	0.0	0.470	10.7	LOS A	3.1	21.4	0.44	0.73	0.58	31.9		
Approa	ach	474	0.0	0.470	8.5	LOS A	3.1	21.4	0.44	0.73	0.58	36.3		
All Veh	icles	1141	2.2	0.470	4.3	NA	3.1	21.4	0.24	0.39	0.30	43.3		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: C:\Users\McLar\Documents\20 09 08 - SIDRA for Chatswood.sip8

Site: 101 [Help Street / Railway Street Future PM]

Help Street / Railway Street Existing Conditions AM Peak Period Site Category: (None) Signals - Eixed Time Isolated - Cycle Ti

Signals - Fixed Time Isolated Cycle Time = 90 seconds (Site Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	l Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South	: Railway	Street											
1	L2	97	28.9	0.143	15.7	LOS B	2.7	22.5	0.50	0.62	0.50	47.0	
2	T1	145	3.4	0.479	20.6	LOS B	8.4	61.1	0.74	0.73	0.74	43.2	
3	R2	133	6.0	0.479	29.7	LOS C	8.4	61.1	0.82	0.76	0.82	40.8	
Appro	ach	375	10.9	0.479	22.6	LOS B	8.4	61.1	0.71	0.71	0.71	43.2	
East:	Help Stre	et											
4	L2	95	12.6	0.186	31.0	LOS C	3.1	24.2	0.78	0.75	0.78	38.8	
5	T1	278	3.6	0.486	28.1	LOS B	10.1	72.9	0.87	0.74	0.87	41.1	
6	R2	80	1.3	0.294	38.0	LOS C	3.1	21.6	0.92	0.76	0.92	36.3	
Appro	ach	453	5.1	0.486	30.4	LOS C	10.1	72.9	0.86	0.74	0.86	39.7	
North:	Railway	Street											
7	L2	148	2.0	0.254	14.4	LOS A	4.9	36.0	0.49	0.60	0.49	48.7	
8	T1	113	11.5	0.747	12.0	LOS A	8.9	66.0	0.54	0.64	0.57	47.7	
9	R2	207	6.3	0.747	35.9	LOS C	8.9	66.0	0.85	0.87	1.01	37.1	
Appro	ach	468	6.2	0.747	23.3	LOS B	8.9	66.0	0.66	0.73	0.74	42.6	
West:	Help Stre	eet											
10	L2	29	3.4	0.488	43.4	LOS D	6.4	45.4	0.95	0.78	0.95	36.0	
11	T1	253	2.0	0.488	38.3	LOS C	6.4	45.4	0.96	0.77	0.96	36.6	
12	R2	8	100.0	0.488	44.9	LOS D	5.6	42.1	0.96	0.77	0.96	35.0	
Appro	ach	290	4.8	0.488	39.1	LOS C	6.4	45.4	0.96	0.77	0.96	36.5	
All Ve	hicles	1586	6.7	0.747	28.0	LOS B	10.1	72.9	0.78	0.74	0.81	40.6	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians												
Mov	-	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m							
P1	South Full Crossing	53	29.7	LOS C	0.1	0.1	0.81	0.81					
P2	East Full Crossing	53	15.1	LOS B	0.1	0.1	0.58	0.58					
P3	North Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P4	West Full Crossing	53	13.4	LOS B	0.1	0.1	0.55	0.55					
All Peo	lestrians	211	24.3	LOS C			0.72	0.72					

Site: 101 [Pacific Highway / Victoria Avenue Future PM]

Pacific Highway / Victoria Avenue Existing Conditions AM Peak Period Site Category: (None) Signals - Eixed Time Coordinated

Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South: Pacific Highway														
2	T1	1958	3.7	1958	3.7	0.513	6.3	LOS A	8.4	60.5	0.49	0.45	0.49	49.9
3	R2	166	5.4	166	5.4	0.558	17.1	LOS B	2.3	16.9	0.83	0.80	0.83	45.7
Appro	bach	2124	3.9	2124	3.9	0.558	7.1	LOS A	8.4	60.5	0.52	0.47	0.52	49.2
East:	Victoria	a Avenue												
4	L2	89	32.6	89	32.6	0.190	30.6	LOS C	1.8	15.9	0.77	0.75	0.77	38.6
6	R2	79	10.1	79	10.1	0.297	41.5	LOS C	1.9	14.5	0.91	0.76	0.91	25.7
Appro	bach	168	22.0	168	22.0	0.297	35.7	LOS C	1.9	15.9	0.83	0.75	0.83	33.4
North	: Pacifi	c Highway												
7	L2	63	36.5	63	36.5	0.702	11.0	LOS A	7.0	51.8	0.39	0.39	0.39	48.9
8	T1	1405	3.5	1405	3.5	0.702	7.6	LOS A	10.3	74.4	0.48	0.45	0.48	50.3
Appro	bach	1468	4.9	1468	4.9	0.702	7.7	LOS A	10.3	74.4	0.48	0.45	0.48	50.3
All Ve	hicles	3760	5.1	3760	5.1	0.702	8.6	LOS A	10.3	74.4	0.52	0.48	0.52	48.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Bacl Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P2	East Full Crossing	53	13.9	LOS B	0.1	0.1	0.56	0.56					
P3	North Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
All Peo	destrians	158	30.8	LOS D			0.81	0.81					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Pacific Highway / Help Street / Fullers Road Future + Network: N101 [Future PM]

Pacific Highway / Help Street / Fullers Road Existing Conditions PM Peak Period Site Category: (None) Signals - Eixed Time Coordinated Cycle Time

Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	Movement Performance - Vehicles													
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	Aver. No.A	verage
ID		Iotai	ΗV	Iotai	ΗV	Sath	Delay	Service	venicies	Distance	Queued	Stop Rate	Cycles a	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Pacifi	c Highway												
1	L2	440	2.3	440	2.3	0.779	22.5	LOS B	12.0	86.1	0.77	0.86	1.03	38.6
2	T1	1513	4.7	1513	4.7	0.779	19.4	LOS B	13.7	99.8	0.79	0.76	0.86	20.6
Appro	ach	1953	4.1	1953	4.1	0.779	20.1	LOS B	13.7	99.8	0.79	0.78	0.90	27.4
East:	Help S	treet												
4	L2	50	16.0	50	16.0	0.169	37.9	LOS C	1.2	9.3	0.88	0.73	0.88	4.5
5	T1	504	5.0	504	5.0	0.794	42.9	LOS D	7.8	54.5	1.00	0.94	1.19	26.3
Appro	bach	554	6.0	554	6.0	0.794	42.4	LOS C	7.8	54.5	0.99	0.92	1.16	25.2
North	: Pacifi	c Highway												
7	L2	8	0.0	8	0.0	0.565	17.0	LOS B	7.6	55.0	0.67	0.61	0.67	12.8
8	T1	1213	3.5	1213	3.5	0.565	13.1	LOS A	7.6	55.0	0.65	0.59	0.65	15.3
9	R2	158	10.1	158	10.1	0.742	20.8	LOS B	2.2	17.0	0.89	0.83	0.99	36.4
Appro	bach	1379	4.2	1379	4.2	0.742	14.0	LOS A	7.6	55.0	0.68	0.62	0.69	21.5
West	Fullers	Road												
10	L2	390	4.1	390	4.1	0.529	25.8	LOS B	8.1	58.5	0.79	0.81	0.79	32.9
11	T1	333	2.7	333	2.7	0.529	27.4	LOS B	8.1	58.5	0.87	0.75	0.87	32.8
12	R2	154	16.2	154	16.2	0.648	47.7	LOS D	4.1	32.5	0.99	0.91	1.33	23.7
Appro	ach	877	5.7	877	5.7	0.648	30.3	LOS C	8.1	58.5	0.85	0.80	0.92	30.8
All Ve	hicles	4763	4.7	4763	4.7	0.794	22.8	LOS B	13.7	99.8	0.79	0.75	0.87	27.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P2	East Full Crossing	53	16.2	LOS B	0.1	0.1	0.60	0.60					
P3	North Full Crossing	53	34.7	LOS D	0.1	0.1	0.88	0.88					
P4	West Full Crossing	53	22.8	LOS C	0.1	0.1	0.71	0.71					
All Peo	lestrians	211	28.3	LOS C			0.78	0.78					

V Site: 101 [Pacific Highway / Mcintosh Street Future PM]

♦♦ Network: N101 [Future PM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Movement Performance - Vehicles													
Mov ID	Turn	Demand F Total	lows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h	
South	South: Pacific Highway														
3a	R1	2012	3.1	2012	3.1	0.470	3.1	LOS A	0.0	0.0	0.00	0.45	0.00	35.6	
Appro	ach	2012	3.1	2012	3.1	0.470	3.1	NA	0.0	0.0	0.00	0.45	0.00	35.6	
East:	East: Mcintosh Street														
4	L2	222	0.0	222	0.0	0.531	7.6	LOS A	0.7	4.8	0.53	0.83	0.77	13.9	
Appro	ach	222	0.0	222	0.0	0.531	7.6	LOS A	0.7	4.8	0.53	0.83	0.77	13.9	
North	East: P	acific Highw	/ay												
24b	L3	43	0.0	43	0.0	0.285	6.5	LOS A	1.8	12.8	0.00	0.51	0.00	43.0	
24a	L1	1543	3.8	1543	3.8	0.285	4.5	LOS A	2.1	14.9	0.00	0.51	0.00	45.3	
Appro	ach	1586	3.6	1586	3.6	0.285	4.5	NA	2.1	14.9	0.00	0.51	0.00	45.3	
All Ve	hicles	3820	3.1	3820	3.1	0.531	3.9	NA	2.1	14.9	0.03	0.50	0.04	40.6	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 101 [Pacific Highway / Railway Street Future PM]

♦♦ Network: N101 [Future PM]

New Site

Site Category: (None) Signals - Fixed Time Coordinated Cycle Time = 90 seconds (Network Site User-Given Phase Times)

Move	Movement Performance - Vehicles													
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	ver. No.A	verage
U		Iotai	ΗV	Iotai	ΗV	Sath	Delay	Service	venicies	Distance	Queuea	Stop Rate	Cycles a	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South: Railway Street														
1b	L3	20	0.0	20	0.0	0.844	50.8	LOS D	6.7	46.9	0.97	0.96	1.29	9.7
3a	R1	446	0.7	446	0.7	0.844	48.8	LOS D	6.7	47.2	0.97	0.96	1.29	26.3
Appro	ach	466	0.7	466	0.7	0.844	48.9	LOS D	6.7	47.2	0.97	0.96	1.29	25.9
North	East: P	acific High	way											
24a	L1	285	3.3	285	3.3	0.624	18.7	LOS B	11.6	83.5	0.71	0.71	0.71	41.0
25	T1	1700	3.3	1700	3.3	0.624	14.4	LOS A	12.0	86.7	0.73	0.67	0.73	40.4
Appro	ach	1985	3.3	1985	3.3	0.624	15.0	LOS B	12.0	86.7	0.73	0.68	0.73	40.5
South	West:	Pacific High	hway											
31	T1	2191	2.0	2191	2.0	0.826	7.9	LOS A	21.4	152.6	0.58	0.55	0.59	50.7
32b	R3	53	32.0	53	32.0	0.284	12.8	LOS A	0.4	3.3	0.27	0.66	0.27	34.8
Appro	ach	2243	2.7	2243	2.7	0.826	8.1	LOS A	21.4	152.6	0.57	0.55	0.58	50.5
All Ve	hicles	4695	2.8	4695	2.8	0.844	15.0	LOS B	21.4	152.6	0.68	0.65	0.71	42.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94				
P8	SouthWest Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94				
All Pe	destrians	105	39.3	LOS D			0.94	0.94				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.